# Cooperative Planning and Environmental Report, Phase 1 

$52^{\text {nd }}$ Avenue South, $45^{\text {th }}$ Street to Sheyenne Street (CR 17)
Fargo-Moorhead Metro COG Project 2017-005
City of Fargo MS-17-AO
NDDOT SU 8-984 (164) PCN 22007

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# A. TECHNICAL MEMORANDUM 1 - EXISTING AND FORECAST CONDITIONS 

Memorandum

## To: $\quad$ 52nd Avenue South Study Review Committee From: Tony Heppelmann, P.E.

Date: January 23, 2018

Re: Technical Memo No. 1
Existing and Forecast Conditions WSB Project No. 10745-000

## Overview

The purpose of the 52nd Avenue South Study is to determine the improvements needed to 52nd Avenue South between 45th Street and Sheyenne Street in the City of Fargo. The 52nd Avenue South corridor is in a rapidly developing area of the City of Fargo. The section of the corridor under study is currently a two-lane rural section with left- and right-turn lanes at major intersections. East of 45th Street, 52nd Avenue South becomes a 4-lane divided arterial with the potential for dual leftturn lanes and right-turn lanes at the major intersections. It becomes a 6-lane section near l-29. The corridor has a wide right-of-way and access is primarily limited to other public streets. This study will determine the number of through and turn lanes needed on 52nd Avenue South, improvements that may be required on cross-streets and the required intersection control. This memo discusses the existing traffic conditions in the corridor, the forecast 2040 travel demand, and the alternatives to accommodate the forecast travel demand at an acceptable level of service.

## Existing Conditions

Functional Classification
The Functional Classification map approved in 2008, classifies 52nd Avenue South as a Minor Arterial within the study area. East of I-29 52nd Avenue South becomes a Principal Arterial. 45th Street is a minor arterial between 52nd Avenue South and 32nd Avenue South. North of 32nd Avenue South, 45th Street is a Principal Arterial. 170th Avenue (Sheyenne Street) is the only other

Minor Arterial intersecting 52nd Avenue within the study corridor. Local roadways intersecting with 52nd Avenue in the study area include 47th Street, 53rd Street, Veterans Blvd, 53rd Avenue, 68th Street, 63rd Street and McMahon Estates Circle.

Arterial roadways are intended to provide the highest level of service at the greatest speed for the longest uninterrupted distance. Their primary function is mobility and therefore some access control is required. Local streets are intended to primarily serve as access to adjacent development.

## Speeds

Figure 2 shows the posted speeds for the roadways within the study area. The posted speed on 52nd Avenue South within the study area is 45 mph .

## Traffic Volumes-Maps and Tables

The 2015 ADT's are shown on Figure 3. The traffic volumes on 52nd Avenue South increase significantly from the west end of the corridor to the east end of the corridor. Currently heavy commercial traffic is about $4 \%$ of the total traffic on 52 nd Avenue South.

Peak hour turning movement volumes were collected in September 2017 at all the intersecting roadways on 52nd Avenue South for both the am and pm peak hour. The counts were taken using a video recording of traffic on September 6 and 7, 2017. The peak hour turning movements are shown on Figure 4.

The existing lane configurations and intersection control at the intersections on 52nd Avenue South are shown on Figure 5. At the time the traffic counts were taken the only signalized intersection in the study area was at 45th Street and 52nd Avenue South. Since then a temporary traffic signal was installed at 63rd Street and 52nd Avenue South. The analysis of the existing conditions assumes an unsignalized intersection at 63rd Street and 52nd Avenue South. The intersection of Sheyenne Street and 52nd Avenue South is a roundabout. The other intersections have stop signs on the intersecting side streets.

## Level of Service

A SimTraffic simulation of the existing traffic conditions (existing traffic volumes and traffic control) was completed for the study area. For the roundabout, Highway Capacity Manual $6^{\text {th }}$ Edition procedure was used. The results of this analysis are summarized in Tables 1 and 2. Figure 6 provides a visual summary of the 2017 AM existing level of service. Figure 7 provides a visual summary of the 2017 PM existing level of service. In general, 52nd Avenue South is currently providing a high level of service for traffic on 52 nd Avenue South. However, there are fairly long delays on some of the side streets during the peak hours at the unsignalized locations including the
northbound approach at 63rd Street, the northbound approach at Veterans Boulevard, and the southbound approach at 53rd Street. Since this analysis was completed a temporary traffic signal was installed at the intersection of 63rd Street and 52nd Avenue South.

Table 1: Existing 2017 Measures of Effectiveness (MOE) - AM Peak Hour

| 은 <br> Oi | Location | Appr | Total Delay by Movement |  |  | LOS by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queueing (feet) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Left-Turn | Through |  |  | Right-Turn |  |  |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave | Max Queue | Storage | $\begin{array}{\|c\|} \hline \text { Ave } \\ \text { Queue } \end{array}$ | Max Queue | Storage | Ave Queue | Max Queue | Storage |
|  | 1: Sheyenne St \& 52nd Avenue S (Based on HCM) | NB | N/A |  |  |  |  |  |  |  | 25 | D |  | 14 | B | NB |  |  |  |  | 208 |  |  |  |  |
| $\begin{array}{\|l\|} \mathbf{0} \\ \frac{\mathrm{O}}{\mathrm{o}} \end{array}$ |  | WB |  |  |  |  |  |  | 6 | A | WB |  |  |  |  |  |  | 25 |  |  |  |  |
| $\frac{0}{\overline{0}}$ |  | SB |  |  |  |  |  |  | 6 | A | SB |  |  |  |  |  |  | 26 |  |  |  |  |
|  |  | EB |  |  |  |  |  |  | 9 | A | EB |  |  |  |  |  |  | 57 |  |  |  |  |
|  | 2: 63rd St \& 52nd Avenue S | NB | 86 | 71 | 78 | F | F | F | 79 | F |  |  | NB |  |  |  | 236 | 530 |  |  |  |  |
|  |  | WB | 6 | 1 | 0 | A | A | A | 2 | A |  |  | WB | 33 | 80 | 275 |  |  |  |  |  |  |
|  |  | SB | 20 | 0 | 4 | C | A | A | 11 | B |  |  | SB |  |  |  | 13 | 42 |  | 12 | 44 | 165 |
|  |  | EB | 4 | 5 | 3 | A | A | A | 5 | A |  |  | EB | 5 | 40 | 300 |  |  |  |  | 4 | 300 |
|  | 3: 53rd Ave S \& 52nd Avenue S | NB | 27 | 0 | 14 | D | A | B | 15 | C |  |  | NB | 30 | 83 |  |  |  |  |  |  |  |
|  |  | WB | 8 | 1 | 0 | A | A | A | 2 | A |  |  | WB | 14 | 49 | 200 |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |  |  |  |
|  |  | EB | 0 | 2 | 1 | A | A | A | 2 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
|  | 4: Veterans Blvd \& 52nd Avenue S | NB | 0 | 44 | 8 | A | E | A | 14 | B |  |  | NB |  |  |  | 4 | 26 |  |  |  |  |
|  |  | WB | 9 | 2 | 1 | A | A | A | 2 | A |  |  | WB | 6 | 32 | 300 |  |  |  | 1 | 17 | 300 |
|  |  | SB | 25 | 17 | 3 | D | C | A | 12 | B |  |  | SB | 21 | 64 | 200 | 4 | 28 |  | 20 | 52 | 200 |
|  |  | EB | 4 | 2 | 1 | A | A | A | 2 | A |  |  | EB | 22 | 55 | 275 |  |  |  |  |  |  |
|  | 5: 53rd St \& 52nd AvenueS | NB | 19 | 25 | 17 | C | D | C | 19 | C |  |  | NB |  |  |  | 17 | 56 |  |  |  |  |
|  |  | WB | 10 | 2 | 2 | B | A | A | 2 | A |  |  | WB | 10 | 43 | 150 |  |  |  |  |  |  |
|  |  | SB | 34 | 25 | 62 | D | D | F | 34 | D |  |  | SB |  |  |  | 42 | 122 |  |  |  |  |
|  |  | EB | 4 | 3 | 1 | A | A | A | 3 | A |  |  | EB | 3 | 29 | 175 |  |  |  |  | 7 | 175 |
|  | 6: 47th St \& 52nd Avenue S | NB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 3 | 3 | A | A | A | 3 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 27 | 0 | 9 | D | A | A | 18 | C |  |  | SB | 44 | 111 |  |  |  |  |  |  |  |
|  |  | EB | 7 | 4 | 0 | A | A | A | 4 | A |  |  | EB | 8 | 34 | 125 |  |  |  |  |  |  |
|  | 7: 52nd Avenue S \& 45th St | NB | 0 | 0 | 0 | A | A | A | 0 | A | 16 | B | NB |  |  |  |  |  |  |  |  |  |
| $\stackrel{8}{\stackrel{\rightharpoonup}{N}}$ |  | WB | 0 | 11 | 5 | A | B | A | 9 | A |  |  | WB |  |  |  | 79 | 165 |  | 42 | 87 | 250 |
| $\begin{array}{\|l\|} \hline \stackrel{\mathrm{x}}{\mathrm{o}} \end{array}$ |  | SB | 25 | 0 | 5 | C | A | A | 22 | C |  |  | SB | 99 | 168 |  |  |  |  | 24 | 58 |  |
|  |  | EB | 30 | 16 | 0 | C | B | A | 18 | B |  |  | EB | 82 | 187 | 250 | 193 | 341 |  |  |  |  |

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

Table 2: Existing 2017 Measures of Effectiveness (MOE) - PM Peak Hour

|  | Location | Appr | Total Delay by Movement |  |  | LOS by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queueing (feet) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  |  |  |  | Left-Turn | Through |  |  | Right-Turn |  |  |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS | Ave | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Storage | Ave Queue | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Storage | Ave Queue | $\begin{array}{c\|} \hline \text { Max } \\ \text { Queue } \end{array}$ | Storage |
|  | 1: Sheyenne St \& 52nd Avenue S (Based on HCM) | NB | N/A |  |  |  |  |  |  |  | 5 | A |  | 9 | A | NB |  |  |  |  | 23 |  |  |  |  |
|  |  | WB |  |  |  |  |  |  | 10 | A | WB |  |  |  |  |  |  | 90 |  |  |  |  |
|  |  | SB |  |  |  |  |  |  | 10 | A | SB |  |  |  |  |  |  | 52 |  |  |  |  |
|  |  | EB |  |  |  |  |  |  | 7 | A | EB |  |  |  |  |  |  | 11 |  |  |  |  |
|  | 2: 63rd St \& 52nd Avenue S | NB | 29 | 28 | 9 | D | D | A | 11 | B |  |  | NB |  |  |  | 49 | 130 |  |  |  |  |
|  |  | WB | 5 | 1 | 1 | A | A | A | 3 | A |  |  | WB | 49 | 117 | 275 |  |  |  |  |  |  |
|  |  | SB | 26 | 19 | 5 | D | C | A | 15 | C |  |  | SB |  |  |  | 18 | 60 |  | 10 | 31 | 165 |
|  |  | EB | 5 | 2 | 1 | A | A | A | 2 | A |  |  | EB | 3 | 23 | 300 |  |  |  |  | 12 | 300 |
|  | 3: 53rd Ave S \& 52nd Avenue S | NB | 19 | 0 | 4 | C | A | A | 5 | A |  |  | NB | 24 | 55 |  |  |  |  |  |  |  |
|  |  | WB | 4 | 2 | 0 | A | A | A | 2 | A |  |  | WB | 6 | 30 | 200 |  |  |  |  |  |  |
|  |  | SB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | SB |  |  |  |  |  |  |  |  |  |
|  |  | EB | 0 | 1 | 0 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
|  | 4: Veterans Blvd \& 52nd Avenue S | NB | 50 | 22 | 8 | F | C | A | 26 | D |  |  | NB |  |  |  | 13 | 43 |  |  |  |  |
|  |  | WB | 4 | 3 | 2 | A | A | A | 3 | A |  |  | WB | 2 | 28 | 300 |  |  |  | 1 | 11 | 300 |
|  |  | SB | 47 | 28 | 15 | E | D | C | 27 | D |  |  | SB | 50 | 121 | 200 | 4 | 31 |  | 43 | 110 | 200 |
|  |  | EB | 10 | 1 | 0 | B | A | A | 2 | A |  |  | EB | 26 | 85 | 275 |  |  |  |  |  |  |
|  | 5: 53rd St \& 52nd Avenue S | NB | 25 | 31 | 10 | D | D | B | 12 | B |  |  | NB |  |  |  | 29 | 73 |  |  |  |  |
|  |  | WB | 6 | 4 | 4 | A | A | A | 4 | A |  |  | WB | 7 | 28 | 150 |  |  |  |  |  |  |
|  |  | SB | 47 | 48 | 34 | E | E | D | 47 | E |  |  | SB |  |  |  | 62 | 163 |  |  |  |  |
|  |  | EB | 7 | 2 | 0 | A | A | A | 2 | A |  |  | EB | 6 | 29 | 175 |  |  |  |  |  |  |
|  | 6: 47th St \& 52nd Avenue S | NB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 6 | 5 | A | A | A | 6 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 43 | 0 | 23 | E | A | C | 32 | D |  |  | SB | 43 | 102 |  |  |  |  |  |  |  |
|  |  | EB | 11 | 3 | 0 | B | A | A | 3 | A |  |  | EB | 10 | 33 | 125 |  |  |  |  |  |  |
|  | 7: 52nd Avenue S \& 45th St | NB | 0 | 0 | 0 | A | A | A | 0 | A | 28 | C | NB |  |  |  |  |  |  |  |  |  |
| $\left.\begin{array}{\|l\|l} \stackrel{8}{\mathrm{~N}} \\ \bar{T} \end{array} \right\rvert\,$ |  | WB | 0 | 40 | 23 | A | D | C | 35 | D |  |  | WB |  |  |  | 391 | 831 |  | 173 | 394 |  |
| 歌 |  | SB | 31 | 0 | 23 | C | A | C | 28 | C |  |  | SB | 119 | 196 |  |  |  |  | 93 | 179 |  |
|  |  | EB | 27 | 8 | 0 | C | A | A | 11 | B |  |  | EB | 53 | 113 | 250 | 91 | 186 |  |  |  |  |

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

## Safety

Figure 8 shows the crashes that have occurred in the corridor over the last 5 years from September 1, 2012 to August 31, 2017. Overall the crash rate on 52nd Avenue South is relatively low. The greatest number of crashes have occurred at the 52nd Avenue South and Sheyenne roundabout. Two-thirds of these crashes are property damage only and the remaining crashes involved minor injuries. There were no fatalities or serious injury crashes in the corridor.

## Forecast Traffic Volumes

Future traffic volumes in this corridor are dependent on the level of development that occurs within the corridor travel shed as well as within the larger metropolitan area. A Travel Demand Model was developed for the Fargo-Moorhead Metropolitan area that models future travel demand based on forecasts of households and employment for the region. The current model was developed for the base year of 2010 and forecast traffic for 2040. The 2010 Base year model was calibrated to estimate 2010 traffic volumes on the 2010 roadway network using the 2010 household and employment information. To develop the 2040 Travel Demand Model, the base year calibrated 2010 model was modified to include 2040 household and employment forecasts and committed changes in the transportation network.

The Advanced Traffic Analysis Center (ATAC) is currently in the process of updating the FargoMoorhead Travel Demand Model to 2045. The updated model will have a calibrated base year model for 2015 and forecast demand for 2045. This new model was not available to meet the schedule for this project. However, the 2045 household and employment forecasts are complete and are used in developing 2040 forecasts for this corridor. The following discussion provides information regarding differences in the 2040 and 2045 household and employment data and how that affects traffic growth in this corridor.

## Socio-Economic Data

Figure 9 compares the household and employment forecasts for 2040 and 2045 for the zones adjacent to the 52nd Avenue South Corridor. In some zones, the 2045 household and employment forecasts are higher and in other zones they are lower than the 2040 forecasts. Although the overall numbers are similar, the 2045 forecasts do represent another 5 years of growth in the region. Figure 10 compares the 2040 and 2045 trip generation for the same zones.

Table 3 shows a comparison of the 2040 and 2045 data for the entire Fargo-Moorhead metropolitan area. Again, the 2045 forecasts are higher than the 2040 forecasts, but because 2045 includes five more years of growth the actual 2045 annual growth rate is slightly lower than the 2040 annual growth rate.

Table 3: Comparison of Socio-Economic Data

| Scenario | Households | Jobs |
| :--- | :--- | :--- |
| Year 2010 | 77,298 | 119,696 |
| Year 2015 | 87,421 | 123,596 |
| Year 2040 Forecast | 112,553 | 163,889 |
| Year 2045 Forecast | 123,087 | 178,232 |

## Model Outputs

2010 Model versus 2010 counts
Figure 11 compares the 2010 model assignments with the 2010 counts. This information indicates how well the model is calibrated to the base year data around 52 nd Avenue South. As can be seen from the comparison the travel demand model for this area is calibrated very well to the actual counts.

## 2040 Model ADT versus 2045 Model ADT

WSB created a 2045 model run using the 2040 existing plus committed transportation network and the 2045 socio-economic data. Figure 12 compares the assigned daily traffic volumes from the 2045 model run with the assigned daily traffic volumes from the 2040 model on the existing plus committed transportation network. Although the 2045 assignments are slightly higher than the 2040 assignments, they also represent another 5 years of growth.
Figure 13 compares the annual growth rates in traffic for the same two models. This figure shows that the annual traffic growth rate in the 2045 model is slightly less than the annual traffic growth rate in the 2040 model. Figure 14 shows the annual traffic growth rates between 2010 and 2015 for the 52nd Avenue South corridor. As can be seen, the traffic growth rates over the last five years have been much greater than what the models are indicating as an annual growth rate through 2040 or 2045. This can be expected since as the area becomes more developed the traffic growth rate should decline relative to the traffic that is already present. In estimating the traffic volumes in 2040 or for interim years between today and 2040, WSB would recommend assuming a linear growth rate rather than a compounded annual growth rate. This will result in declining annual growth rates over time as the traffic volumes increase.

## Existing Plus Committed network versus Roadway Vision Plan network

Significant changes in the transportation network are expected in this area of the City. Figure 15 shows the additional transportation improvements that have been identified in the 2014 Fargo Moorhead Long Range Transportation Plan (LRTP) in what is labeled in the LRTP as the Roadway Vision Plan. Changes in the transportation network included in the Roadway Vision Plan network include the extension of $45^{\text {th }}$ Street south of 52 nd Avenue South, the extension of Veterans Boulevard south of 52nd Avenue South, the upgrade and extension of $64^{\text {th }}$ Avenue to the east across I-29, and the upgrade and extension of $76^{\text {th }}$ Avenue to the east with an interchange at I-29. These improvements will have a significant impact on the traffic demand on the 52nd Avenue South corridor. Figure 16 shows the 2040 model assignments for this area with the existing plus committed network. Figure 17 shows 2040 Roadway Vision Plan model assignments. By comparing the two figures it can be seen that ADT's on 52nd Avenue South within the study area are significantly lower on the Roadway Vision Plan network versus the Existing Plus Committed
network. In other words, the other transportation facilities that are constructed in this area will have a significant impact on the forecasts for 52nd Avenue South.

## Fiscally Constrained Network

The current Fargo-Moorhead Long Range Transportation Plan also includes a Fiscally Constrained network. Within the 52nd Avenue South study area this network includes some, but not all, of the improvements in the Roadway Vision Plan network. Figure 18 shows the fiscally constrained network. Some of the key differences from the Roadway Vision Plan network include:

45th Street and Veterans Boulevard are only extended to 64th Avenue versus 76th Avenue.

76th Avenue does not have an interchange with I-29.

The Long-Range Transportation Plan will be updated in 2018.

Based on discussions with the 52nd Avenue South Study Review Committee it was concluded that the fiscally constrained network is probably the most appropriate transportation network to use for developing the 2040 forecasts for the 52nd Avenue South corridor with the exception that it is not expected that Veteran's Boulevard will be extended south to 64th Avenue because of the cost of bridging over the drainage ditch.

## 2040 ADT Forecasts

Figure 19 shows the 2040 ADT forecasts for the corridor. These forecasts are based on the modified fiscally constrained network described above and are less than the ADT's forecast assuming just the existing plus committed network but more than the ADT's forecast with the Roadway Vision Plan network. The 2040 ADT forecasts shown on Figure 19 are based on the 2045 Model Assignments (2045 Socio-economic data) using the modified fiscally constrained network that have been adjusted to 2040 assuming straight line growth. The forecasts have also been adjusted based on the 2017 counts on the lower volume local streets.

Figure 20 shows 2040 am and pm peak hour turning movement forecasts for the intersections in the corridor based on the 2040 forecasts shown on Figure 19. These turning movement forecasts are based on general travel patterns obtained from existing turning movement counts and the travel model. For example, at Sheyenne Street and 52nd Avenue South there is a very heavy northbound to eastbound right-turn movement in the am peak hour and a similar heavy westbound to southbound left-turn movement in the pm peak hour. The 2040 turning movement forecasts reflect this same pattern based on the ADT growth on Sheyenne Street and 52nd Avenue South. Another
example is that existing turning movements and ADT's show a strong orientation towards I-29 with ADT's increasing on 52nd Avenue South from west to east and heavy northbound right-turn movements and southbound left-turn movements onto 52nd Avenue South. The forecast 2040 ADT's also show this same pattern, and this is reflected in the 2040 am and pm peak hour turning movement forecasts.

## Future Traffic Operations

## Overview of Analysis

Future traffic operations will depend on the improvements that are made in the corridor to accommodate the 2040 travel demand. Three alternatives were identified at the corridor level for analysis purposes. These alternatives include the No-build alternative, a 4-Lane Divided roadway with traffic signals, and a 4-lane Divided roadway with roundabouts. Based on the 2040 traffic forecasts there are two intersections in the corridor that are expected to meet warrants for a traffic signal; they are the intersections of 52nd Avenue South/63rd Street and 52nd Avenue South/Veteran's Boulevard. The 45th Street intersection is already signalized, and the Sheyenne Street intersection is currently a roundabout intersection. The 4-Lane Divided with Traffic Signals alternative would create a corridor where the major cross-street traffic is handled at signalized intersections. The 4-Lane Divided with Roundabouts would create a corridor where the major crossstreet traffic is handled with roundabout intersections.

In between the major intersections of 63rd Street, Veterans Boulevard and 45th Street are three lower volume access intersections. These include the intersections of 52nd Avenue South at 53rd Avenue, 52nd Avenue South at 53rd Street and 52nd Avenue South at 47th Street. There are five options at each of these locations; full access, $3 / 4$ access, RCUT, J-Turn, and Right-in, Right-out. The RCUT (Reduced Conflict U-Turn) has direct access for all moves except the north-south throughs and the left-turns from the minor street. These movements will have to take a right-turn and then use the U-turn intersection to go in the opposite direction and either make a right-turn at the intersection (north-south through's) or continue through the intersection (left turn from minor street). The J-Turn intersection is similar to the RCUT intersection except it does not allow the left-in to the minor street. These lefts must use the U-Turn intersection. The last option is to restrict the minor intersections to right-in, right-out. Left-turns and through traffic will have to go to the next intersection and either make a U-turn or use other local streets to access the adjacent development.

A SimTraffic simulation of the 2040 traffic operations with the 2040 forecast turning movements was completed for a No-Build and Build Alternatives for 52nd Avenue South. The roundabout at Sheyenne Street was analyzed using Highway Capacity Manual $6^{\text {th }}$ Edition procedures. The No-

Build alternative assumes that no improvements are made in the 52nd Avenue South corridor except at 45th Street, which is assumed to be extended south to 64th Avenue as a four-lane divided roadway. The No-build alternative assumes a traffic signal at 63rd Street since a temporary signal has been installed at this location.

## 2040 No-Build Conditions

Tables 4 and 5 show the results of the traffic operations analysis for the No-build alternative for the 2040 forecast am and pm peak hours. Below is a discussion of the operations expected at each intersection moving from west to east.
Sheyenne Street and 52nd Avenue South - This roundabout is forecast to operate at LOS "F" in the am peak hour and LOS "E" in the pm peak hour. The worst approaches are the northbound approach in the am peak hour and the southbound and westbound approaches in the pm peak hour.

63rd Street and 52nd Avenue South - This is assumed to be a signalized intersection in the NoBuild alternative. This intersection will operate at LOS "D" in the am peak hour and at LOS "B" in the pm peak hour.

53rd Avenue and 52nd Avenue South - This is a low volume unsignalized intersection in the NoBuild alternative that will operate at an acceptable LOS "D" or better in the am and pm peak hours. It is possible that the north leg of this intersection could have higher volumes than forecast for 2040 if higher traffic generating commercial uses are developed on this currently undeveloped property.

Veteran's Boulevard and 52nd Avenue South - This is assumed to be an unsignalized intersection in the No-Build alternative and the southbound left turns are at LOS "F" in the am peak hour and the entire southbound approach is at LOS "F" in the pm peak hour. 53rd Street and 52nd Avenue South - This is assumed to be an unsignalized intersection in the NoBuild alternative and thru and left-turn movements on the northbound and southbound approaches are at LOS "F" in the am and pm peak hour.

47th Street and 52nd Avenue South - This is assumed to be an unsignalized intersection in the NoBuild alternative and the southbound approach is at LOS "F" in the am and pm peak hours. There is currently no south leg on this intersection but is planned to accommodate future development.

45th Street and 52nd Avenue South - This intersection is currently signalized and is forecast to operate at LOS "D" in the am and pm peak hours.

In general, significant side street delays as well as back-ups on 52nd Avenue South can be expected with 2040 traffic and no major improvements on 52nd Avenue South or side streets.

Table 4: No-Build 2040 Measures of Effectiveness (MOE) - AM Peak Hour

| $\overline{0}$ <br> 0 <br> 0 <br> 0 |  | Appr | Total Delay by Movement |  |  | LOS by Movement |  |  |  | LOS by Approach (Sec/Veh) |  | LOS by <br> Intersection <br> (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queueing (feet) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Location |  |  |  |  | Left-Tur |  |  | Throug |  |  |  |  |  | Right-T | urn |  |
|  |  |  | $\square$ | T |  |  |  |  |  | L |  |  |  |  | ${ }^{R}$ | Delay | LOS | Delay | LOS | Ave <br> Queue | Max Queue | Storag e | Ave <br> Queue | Max <br> Queue | Storag e | Ave <br> Queue | Max <br> Queue | $\begin{array}{\|l\|} \hline \text { Storag } \\ \text { e } \\ \hline \end{array}$ |
|  | 1: Sheyenne St \& 52nd Avenue S (Based on HCM) | NB | N/A |  |  |  |  |  |  | 245 | F | 114 | F |  | NB |  |  |  |  | 1281 |  |  |  |  |
|  |  | WB |  |  |  |  |  |  |  |  | C |  |  | WB |  |  |  |  | 158 |  |  |  |  |
|  |  | SB |  |  |  |  |  |  |  |  | B |  |  | SB |  |  |  |  | 124 |  |  |  |  |
|  |  | EB |  |  |  |  |  |  |  | 14 | B |  |  | EB |  |  |  |  | 73 |  |  |  |  |
|  | 2: 63rd St \& 52nd Avenue S | NB | 29 | 34 |  | C |  | C | C | 26 | C | 21 | C | NB | 35 | 86 | 100 | 13 | 59 |  | 156 | 323 |  |
|  |  | WB |  |  |  | B | B | - A | A | 12 | B |  |  | WB | 62 | 144 | 275 | 84 | 218 |  | 2 | 17 | 275 |
|  |  | SB |  | 0 | 6 | C | A | A | A | 17 | B |  |  | SB | 27 | 75 | 100 | 19 | 64 |  |  |  |  |
|  |  | EB | 16 | 26 | 8 | B | C | C A | A | 24 | C |  |  | EB | 32 | 77 | 300 | 195 | 325 |  | 17 | 111 | 300 |
| $\begin{aligned} & \circ \\ & \stackrel{\circ}{2} \\ & \stackrel{0}{01} \\ & \frac{2}{2} \\ & \mathfrak{F} \end{aligned}$ | 3: 53rd Ave S/New 53rd Ext \& 52nd Avenue S | NB | 38 | 35 | 13 | E | E | E B | B | 15 | C |  |  | NB |  |  |  | 27 | 88 |  |  |  |  |
|  |  | WB |  | 2 | 1 | A | A | A | A | 2 | A |  |  | WB | 18 | 55 | 200 |  |  |  |  |  |  |
|  |  | SB | 31 | 26 | 13 | D | D | B | B | 24 | C |  |  | SB |  |  |  | 22 | 69 |  |  |  |  |
|  |  | EB | 3 | 2 | 1 | A | A | A A | A | 2 | A |  |  | EB | 13 | 33 | 250 |  | 4 |  |  |  |  |
| $\circ$ <br> $\stackrel{\circ}{0}$ <br> $\stackrel{1}{2}$ <br> $\stackrel{2}{5}$ <br> -1 | 4: Veterans Blvd \& 52nd Avenue S | NB | 0 | 0 | 7 | A | A | A A | A | 5 | A |  |  | NB |  |  |  | 3 | 22 |  |  |  |  |
|  |  | WB | 5 | 3 | 2 | A | A | A | A | 3 | A |  |  | WB | 8 | 33 | 300 |  | 4 |  | 3 | 24 | 300 |
|  |  | SB | 111 | 31 | 9 | F | D | A | A | 47 | E |  |  | SB | 89 | 208 |  | 20 | 121 |  | 34 | 113 | 200 |
|  |  | EB | 8 | 2 | 1 | A | A | A A | A | 3 | A |  |  | EB | 40 | 98 | 275 |  |  |  |  |  |  |
| $\circ$ <br> $\stackrel{\circ}{0}$ <br> $\stackrel{0}{0}$ <br> $\stackrel{2}{2}$ <br> $\stackrel{5}{5}$ | 5: 53rd St \& 52nd Avenue S | NB | 44 | 55 | 25 | E | F | - D | D | 33 | D |  |  | NB |  |  |  | 27 | 76 |  |  |  |  |
|  |  | WB | 10 | 3 | 2 | B | A | A A | A | 3 | A |  |  | WB | 18 | 48 | 150 |  |  |  |  |  |  |
|  |  | SB | 519 | 503 | 480 | F | F | F F | F | 518 | F |  |  | SB |  |  |  | 441 | 557 |  |  |  |  |
|  |  | EB | 5 | 4 | 2 | A | A | A A | A | 4 | A |  |  | EB | 5 | 30 | 175 |  |  |  |  | 3 | 175 |
| Thru-Stop | 6: 47 th St \& 52nd Avenue S | NB | 0 | 0 | 0 | A | A | A A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 6 | 5 | A | A | A A | A | 6 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 164 | 0 | 134 | F | A | A F | F | 148 | F |  |  | SB |  |  |  |  |  |  |  |  |  |
|  |  | EB | 11 | 5 | 0 | B | A | A A | A | 5 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
|  | 7: 45th St \& 52nd Avenue S | NB | 35 | 36 | 23 | D | D | C | C | 31 | C | 38 | D | NB | 92 | 208 | 300 | 162 | 264 |  | 135 | 285 | 300 |
|  |  | WB | 69 | 91 | 48 | E | F | F D | D | 72 | E |  |  | WB | 118 | 350 |  | 525 | 1133 |  | 211 | 394 |  |
|  |  | SB | 40 | 26 | 11 | D | C | C B | B | 30 | C |  |  | SB | 113 | 204 | 500 | 105 | 173 |  | 37 | 88 |  |
|  |  | EB | 31 | 24 | 21 | C | C | C C | C | 26 | C |  |  | EB | 83 | 150 | 250 | 157 | 252 |  |  |  |  |

*Roundabout MOE based on HCM 6 ${ }^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

Table 5: No-Build 2040 Measures of Effectiveness (MOE) - PM Peak Hour

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic 2040 Build Alternative Conditions

## 4-Lane Divided with Traffic Signals

The assumed intersection lane configuration for the 2040 4-Lane Divided with Traffic Signal alternative is shown on Figure 21. Tables 6 and 7 shows the results of the traffic operations analysis for this alternative for the 2040 forecast am and pm peak hours. Below is a discussion of the operations expected at each intersection moving from west to east.
Sheyenne Street and 52nd Avenue South - Because this roundabout is forecast to operate at LOS " $F$ " and LOS " $E$ " in the am and pm peak hours in 2040 under the No- build alternative, improvements to the roundabout were identified to provide acceptable traffic operations. The improvements required include making the north and south legs of the roundabout two-lane approaches with two lanes through the roundabout in the northbound and southbound directions. The northbound and southbound right lanes would be combined right-thru lanes and the left lanes would be combined left-thru lanes. The other improvement would be the addition of a right-only lane on the east approach to the
roundabout. With the improvements, the roundabout is expected to operate at LOS "A" in the am peak hour and LOS "A" in the pm peak hour.

63rd Street and 52nd Avenue South - This is assumed to be a signalized intersection in the Build alternative and is expected to operate at LOS " B " in the am and pm peak hours.
53rd Avenue and 52nd Avenue South - This is a low volume unsignalized intersection in this alternative. The southbound and northbound left-turns and throughs are expected to operate at an acceptable LOS "D" or better in the am and pm peak hours with the 2040 forecasts. However, any volumes higher than forecast are likely to result in unacceptable delays for the northbound and southbound left-turns and through movements.

Veteran's Boulevard and 52nd Avenue South - This is assumed to be a signalized intersection in the Build alternative and is expected to operate at LOS " B " in the am and pm peak hours.

53rd Street and 52nd Avenue South - This is assumed to be an unsignalized intersection in the Build alternative and the southbound approach is expected to operate at LOS " F " in the am and pm peak hours. The northbound through and left-turn movements are expected to operate at LOS "E" and "F".

47th Street and 52nd Avenue South - This is assumed to be an unsignalized intersection in the Build alternative and the southbound left-turn is LOS "F" in the am peak hour and LOS " $E$ " in the pm peak hour. There is currently no south leg on this intersection but is planned to accommodate future development.

45th Street and 52nd Avenue South - This intersection is currently signalized and is forecast to operate at LOS "C" in the am and pm peak hours with this alternative.

Table 6: Build 2040 4-Lane Divided with Signals - Full Access - AM Peak Hour

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

Table 7: Build 2040 4-Lane Divided with Signals - Full Access - PM Peak Hour

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

## RCUT or 3/4 Access at Intersections at Minor Streets

Tables 8 and 9 show the traffic operations results if RCUT's or $3 / 4$ Access are constructed at the three minor street approaches rather than full access with side street stop conditions. The analysis assumes traffic signals at $63^{r d}$ Street and at Veteran's Boulevard but the roundabout alternative would have similar results. These side street intersections would now operate at LOS " $A$ ". The RCUT intersection does create some additional travel time and distance since the minor street through and left turn movements need to travel about 1000 feet further. This would add approximately 25 seconds of additional travel time for these movements. However, this is more than offset by the reduced delay on the side street. With the $3 / 4$ Access intersection the minor street throughs and left turns would have to travel to the next intersection to make a U- turn or take the local streets to a signalized intersection. The additional travel time related to using these routes will in many cases be greater than the delay that is forecast on the side street approach.

Table 8: Build 2040 4-Lane Divided with Signals - RCUT or $3 / 4$ Access- AM Peak Hour


Table 9: Build 2040 4-Lane Divided with Signals - RCUT or 3/4 Access - PM Peak Hour


## J-Turn Intersections at Minor Streets

Tables 10 and 11 show the traffic operations results if J-turns are constructed at the three minor street approaches rather than full access with side street stop conditions. The analysis assumes traffic signals at 63rd Street and at Veteran's Boulevard but the roundabout alternative would have similar results. These side street intersections operate at LOS "A" with J-turns. The J-turn intersection will create some additional travel time and distance for the left-turn in as well as for the minor street through and left turn movements. These movements will need to travel about 1000 feet further. This would add approximately 25 seconds of additional travel time for these movements. For the left-turn in at these intersections the additional travel distance and time would be greater than delay for this left-turn with the RCUT intersection. For the side street left-turns and through movements the additional travel distance and travel time is more than offset by the reduced delay on the side street.

Table 10: Build 2040 4-Lane Divided with Signals - J-Turn- AM Peak Hour


Table 11: Build 2040 4-Lane Divided with Signals - J-Turn- PM Peak Hour


## Right-in, Right-out Intersections at Minor Streets

Tables 12 and 13 show the traffic operations results if right-in and right-out intersections are constructed at the three minor street approaches rather than full access with side street stop conditions. The analysis assumes traffic signals at 63rd Street and at Veteran's Boulevard but the roundabout alternative would have similar results. These side street intersections operate at LOS " A " with right-in and right-out intersections. The right-in and right-out intersection will create significant additional travel time and distance for the left-turn in as well as for the minor street through and left turn movements. These movements will need to travel to the next intersection to make a U-turn or take the local streets to a signalized intersection. The additional travel time related to using these routes will in many cases be greater than the delay that is forecast on the side street approach.

Table 12: Build 2040 4-Lane Divided with Signals - RIRO- AM Peak Hour

| Intersection |  | Appr | Total Delay by Movement |  | LOS by Movement |  |  | LOS by Approach (Sec/Veh) |  | LOS by Intersection (Sec/Veh) |  | Appr | Average \& |  |  | Maximum Traffic Queu eing (feet) |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Location |  |  |  | Left-Tu |  |  |  |  | Throug |  |  |  | Right-T | urn |  |
|  |  |  |  | $\mathrm{T}^{\top}$ R |  |  |  | L |  |  |  | R | Delay | LOS | Delay | LOS | Ave <br> Queue | $\begin{aligned} & \text { Max } \\ & \text { Queue } \end{aligned}$ | Storage | Ave Queue | $\begin{aligned} & \text { Max } \\ & \text { Queue } \end{aligned}$ | Storage | Ave <br> Queue | Max | Storage |
|  | 1: Sheyenne St \& 52nd Avenue S (Based on HCM) | NB | N/A |  |  |  |  | 10 | B |  | A |  | NB |  |  |  |  | 146 |  |  |  |  |
|  |  | WB |  |  |  |  |  |  |  |  |  | WB |  |  |  |  | 46 |  |  |  |  |
|  |  | SB |  |  |  |  |  | 4 | A |  |  | SB |  |  |  |  | 32 |  |  |  |  |
|  |  | EB |  |  |  |  |  | 11 | B |  |  | EB |  |  |  |  | 57 |  |  |  |  |
|  | 2: 63rd St \& 52nd Avenue S | NB | 19 | 19 19 | B | B | B | 15 | B | 17 | B | NB | 28 | 69 | 150 | 8 | 34 |  | 108 | 229 | 300 |
|  |  | WB | 20 | $15 \quad 2$ | C | B | A | 16 | B |  |  | WB | 79 | 152 | 275 | 77 | 160 |  | 4 | 22 | 275 |
|  |  | SB | 19 | 193 | B | B | A | 10 | B |  |  | SB | 23 | 72 | 165 |  | 8 |  | 15 | 42 | 150 |
|  |  | EB | 14 | 224 | B | C | A | 20 | C |  |  | EB | 34 | 87 | 300 | 127 | 224 |  | 12 | 44 | 300 |
|  | 3: 53rd Ave S/New 53rd Ext \& 52nd Avenue S | NB | 0 | 08 | A | A | A | 8 | A |  |  | NB |  |  |  |  |  |  | 24 | 60 |  |
|  |  | WB |  | $3{ }^{3}$ | A | A | A | 3 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
| $\bigcirc$ |  | SB | 0 | $0 \quad 4$ | A | A | A | 4 | A |  |  | SB |  |  |  |  |  |  | 17 | 37 |  |
| 亮 |  | EB | 0 | 10 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
|  | 4: Veterans Blvd \& 52nd Avenue S | NB | 19 | 15 5 | B | B | A | 9 | A | 13 | B | NB |  | 4 | 100 | 2 | 21 |  |  |  |  |
|  |  | WB | 14 | 134 | B | B | A | 12 | B |  |  | WB | 78 | 152 | 300 | 77 | 157 |  | 37 | 86 | 300 |
|  |  | SB | 27 | 276 | C | C | A | 14 | B |  |  | SB | 44 | 122 | 200 | 6 | 39 |  | 32 | 70 | 200 |
|  |  | EB | 13 | 13 3 | B | B | A | 13 | B |  |  | EB | 72 | 162 | 275 | 106 | 208 |  | 1 | 12 | 275 |
|  | 5: 53rd St \& 52nd Avenue S | NB | 0 | 06 | A | A | A | 6 | A |  |  | NB |  |  |  |  |  |  | 19 | 51 |  |
|  |  | WB | 0 | 10 | A | A | A | 1 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 0 | $0{ }^{0}$ | A | A | A | 6 | A |  |  | SB |  |  |  |  |  |  | 35 | 86 |  |
|  |  | EB | 0 | $1{ }^{1}$ | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
| $\begin{aligned} & \circ \\ & \stackrel{\circ}{2} \\ & \stackrel{0}{0} \\ & \frac{2}{2} \\ & \frac{1}{2} \end{aligned}$ | 6: 47th St \& 52nd AvenueS | NB | 0 | 0 | A | A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 44 | A | A | A | 4 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 0 | 08 | A | A | A | 8 | A |  |  | SB |  |  |  |  |  |  | 45 | 101 |  |
|  |  | EB | 0 | 10 | A | A | A | 1 | A |  |  | EB |  |  |  |  |  |  |  |  |  |
|  | 7: 45th St \& 52nd Avenue S | NB | 28 | $37 \quad 21$ | C | D | C | 30 | C | 27 | c | NB | 75 | 161 | 300 | 157 | 245 |  | 127 | 251 | 300 |
|  |  | WB | 24 | 3315 | C | C | B | 25 | C |  |  | WB | 33 | 73 | 250 | 115 | 189 |  | 91 | 212 | 250 |
|  |  | SB | 31 | $26 \quad 5$ | C | C | A | 26 | C |  |  | SB | 109 | 181 | 500 | 107 | 189 |  | 26 | 73 |  |
|  |  | EB | 31 | 288 | C | C | A | 27 | C |  |  | EB | 103 | 184 | 250 | 154 | 253 |  | 22 | 68 | 250 |

Table 13: Build 2040 4-Lane Divided with Signals - RIRO- PM Peak Hour


## 4-Lane Divided with Roundabouts

Tables 14 and 15 shows the results of the traffic operations analysis for the 4-lane divided with roundabout alternative with the 2040 forecast am and pm peak hours. In general, this alternative operates like the 4-Lane Divided with Traffic Signals. The roundabouts are expected to result in slightly less delay and better level of service at 63rd Street and at Veterans Drive.

The three access intersections with side street stops would operate like the 4-lane divided with signals alternative. The RCUT, J-Turn and Right-In Right-Out options for minor intersections were also evaluated under this option and the MOE's were found to be like the signal scenario. The MOE's for these scenarios are shown in Tables 18 thru 23.

Table 14: Build 2040 4-Lane Divided with Roundabouts -Full Access- AM Peak Hour


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Table 15: Build 2040 4-Lane Divided with Roundabouts - Full Access- PM Peak Hour


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Daon 19

## Summary of Traffic Operations for 2040 Build Alternatives

Table 16 summarizes the level of service results for each of the alternatives. In general, the table shows the deficiencies of the existing facility to serve the forecast 2040 traffic volumes. It also shows the poor level of service expected for the side streets with full access at the minor access intersections. The RCUT design addresses the delays expected for the minor street throughs and left-turns. This is also addressed by the J-Turn, 3/4 Access and Right-in and Right-out alternatives. However, these options do create additional travel distance and time without any noticeable reduction in delays.

Table 16: Summary of 2040 Intersection Levels of Service by Alternative

|  |  | Alternatives |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Intersection | Traffic Control | No-B |  | 4-Lane with <br> Signals/Full Access |  | 4-Lane with <br> Signals /RCUT |  | 4-Lane with <br> Signals/3/4 Access |  | 4-Lane with Signals/J-Turn |  | 4-Lane with Signals/RI, RO |  | 4-Lane with Roundabouts /Full Access |  |
|  |  | AM | PM | AM | PM | AM | PM | AM | PM | AM | PM | AM | PM | AM | PM |
| Sheyenne Street | Roundabout | F | E | A | A | A | A | ${ }^{\text {A }}$ | A | ${ }^{\text {A }}$ | A |  | A | A | A |
| 63rd Street | Signal or Roundabout | C | B | B | B | B | B | B | B | B | B | B | B | A | A |
| 53rd Avenue | Full,RCUT,J-Turn, RI-RO | E1/ | D 1/ | D 1/ | E1/ | B 1/ | A 1/ | B 1/ | A 1/ | A 1/ | A 1/ | A 1/ | A 1/ | E1/ | C 1/ |
| Veterans | Signal or Roundabout | F 1/ | F 1/ | A | B | A | B | A | B | B | B | B | B | A | A |
| 53rd Street | Full,RCUT,J-Turn, RI-RO | F 1/ | F 1/ | F 1/ | F 1/ | A 1/ | B 1/ | A 1/ | B 1/ | A 1/ | B 1/ | A 1/ | B 1/ | F 1/ | F 1/ |
| 47th Street | Full,RCUT,J-Turn, RI-RO | F 1/ | F 1/ | F 1/ | D 1/ | A 1/ | A 1/ | A 1/ | A 1/ | A 1/ | A 1/ | A 1/ | B 1/ | F 1/ | E1/ |
| 45th Street | Signal | D 2/ | C2/ | C | c | C | C | C | C | C | C | C | C | C | C |

At side street stop control intersections the reported Level of Service is for the worst side street approach
The westbound through movements are at LOS E and F with only one westbound lane.

## Corridor Travel Times

The table below summarizes the delay, travel time and average speed in traveling the corridor between 45th Street and Sheyenne Street for the different alternatives. The analysis is based on full access at minor street intersections but the other alternatives for the minor street intersections would have similar results. In general, the roundabouts would have slightly more delay and higher travel times than the signal alternative since some of the 52nd Avenue South through traffic will be able to travel through the intersections at 63rd Street and Veteran's Boulevard without slowing or stopping in

Technical Memorandum 1 - Existing and Forecast Year 2040 Projected Conditions
Daso the signal alternative. With roundabouts, all through traffic must slow to go through the intersection resulting in slightly lower average speeds on 52nd Avenue.

## Technical Memorandum 1 - Existing and Forecast Year 2040 Projected Conditions

Table 17: Summary of Corridor Travel Times by Alternative (AM peak hour)

| Alternative | Eastbound |  |  | Westbound |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Delay (sec) | Total Travel <br> Time (sec) | Average <br> Speed (mph) | Delay (sec) | $\begin{aligned} & \text { Total Travel } \\ & \text { Time (sec) } \end{aligned}$ | Average Speed (mph) |
| Existing | 44.4 | 245 | 35 | 25.1 | 208.5 | 43 |
| No-Build | 97.8 | 289.6 | 30 | 93.1 | 282.6 | 32 |
| 4-Lane w/Signals | 91.6 | 282.1 | 31 | 89.2 | 280.6 | 32 |
| 4-Lane w/Roundabouts | 97.7 | 293.8 | 29 | 98.1 | 289.5 | 31 |

## Safety

Roundabouts versus Signals
The most recent crash studies on roundabouts show that a 2-lane by 1-lane roundabout has about the same crash rate as a signalized intersection. However, the fatality and serious injury crash rate for a $2 \times 1$ roundabout is about $50 \%$ less than for a signalized intersection. This is based on the Federal Highway Crash Modification Factors Clearinghouse and consistent with a recently completed study of traffic safety at roundabouts in Minnesota.

## $\underline{\text { R-CUT and J-Turn intersections versus full access side street stop intersections }}$

The Federal Highway Crash Modification Factors Clearinghouse has several studies that indicate an RCUT intersection will reduce the crash rate at a full access side street stop intersection by about $30 \%$ and reduce the overall severity of crashes.
A similar study of the J-turn intersection indicated that the J-turn design resulted in about a 35\% reduction in the overall crash rate with about a $50 \%$ reduction in the crash rate for injury and fatal crashes.

Figure 1: Functional Classification Map


Figure 2: Posted Speeds


Figure 3: 2015 ADT's


Figure 4: 2017 Turning Movements Counts


Figure 5: Existing Intersection Lane Configurations and Control Types


Figure 6: 2017 AM Existing Level of Service


Figure 7: 2017 PM Existing Level of Service


Figure 8: Crashes September 2012 to August 2017


Figure 9: Forecast Households and Jobs - 2040 vs 2045


Figure 10: Forecasted Productions and Attractions - 2040 vs 2045


Figure 11: Comparison of Actual 2010 ADT vs 2010 Model ADT


Figure 12: Comparison of 2040 vs 2045 ADT Assignments on Existing Plus Committed Transportation Network



Figure 14: Annualized Traffic Growth Rate - 2010 to 2015


## CHAPTER 10 - TRANSPORTATION VISION PLAN

Figure 10-1: Roadway Vision Plan


Source: Metro COG


Figure 17: 2040 Roadway Vision Plan Model Assignments


Figure 12-2: Fiscal Constrained Plan


Source: Metro COG

Figure 19: 2040 ADT Forecasts


Figure 20: 2040 AM \& PM Peak Hour Turning Movements


Figure 21: 2040 Build Alternative Intersection Lane Configurations


Figure 22: 2040 No-Build AM Level of Service


Figure 23: 2040 No-Build PM Level of Service


Figure 24: 2040 Build AM Level of Service


Figure 25: 2040 Build PM Level of Service


Table 18: Build 2040 4-Lane Divided with Roundabouts -RCUT- AM Peak Hour


Table 19: Build 2040 4-Lane Divided with Roundabouts -RCUT- PM Peak Hour


Table 20: Build 2040 4-Lane Divided with Roundabouts -J-Turn- AM Peak Hour


Table 21: Build 2040 4-Lane Divided with Roundabouts -J-Turn- PM Peak Hour


Table 22: Build 2040 4-Lane Divided with Roundabouts -RIRO- AM Peak Hour


Table 23: Build 2040 4-Lane Divided with Roundabouts -RIRO- PM Peak Hour


## B TECHNICAL MEMORANDUM 2 - PUBLIC INPUT SUMMARY

Subject: Technical Memorandum 2-52 ${ }^{\text {nd }}$ Ave S Public Input Summary
Date: January 24, 2018
Project: FM MetroCOG - 2017-0057
City of Fargo - MS-17-A0
NDDOT - SU-8-984(164), PCN 22007

## 1) INTRODUCTION

The purpose of the $52^{\text {nd }}$ Ave $S$ Study is to determine a proactive plan for the improvements needed for $52^{\text {nd }}$ Ave S between $45^{\text {th }} \mathrm{St} S$ in the City of Fargo and Sheyenne Street in the City of West Fargo to serve as an important link in the multimodal transportation system. Thereby, accommodating the long-term growth and development in the project area by addressing future capacity and safety needs.

The section of the corridor under study is currently a two-lane rural section with left and right turn lanes at major intersections. East of $45^{\text {th }} \mathrm{St} \mathrm{S}, 52^{\text {nd }}$ Ave $S$ becomes a 4-lane divided arterial with the potential for dual left turn lanes and right turn lanes at the major intersections. $52^{\text {nd }}$ Ave $S$ becomes a 6-lane section near Interstate 29.

Alternatives under consideration include No-Build and Reconstruct to a 4-lane urban corridor. Several intersection options are being studied including: traffic signals, roundabouts, full access stop controlled, $3 / 4$ access, J-turn and R-cut (reduced conflict) intersections, as well as median closures at lower volume intersections.

Three public input meetings are planned throughout the study. The first two meetings were held as part of the planning process on October 26, 2017 and December 21, 2017. The third and final meeting will be held as part of the environmental document phase of the project and is planned for February 2018. This technical memorandum covers Public Input Meetings 1 and 2. Public Input Meeting 3 will be included in the project Documented Categorical Exclusion (DCE).

## 2) PUBLIC INPUT MEETING \#1

### 2.1 MEETING DETAILS

Public Input Meeting \#1 was held at the Calvary United Methodist Church, $457545^{\text {th }}$ Street South, Fargo, ND, on October 26, 2017 from 5:00 PM to 7:00 PM. The meeting was an open house format, with large format aerial
photos of the existing project corridor and side boards showing existing and projected traffic volumes, intersection level of service (LOS) and possible intersection configurations.

The meeting was held to inform the public about the upcoming project as well as gather input on existing issues and proposed improvements.

### 2.2 ATTENDEES

The meeting had a total of 29 attendees including the project team. The project team was represented by Houston Engineering, Inc. (Jeremy McLaughlin, Jeff Lansink and Adam Ruud), WSB and Associates (Tony Heppelman), City of Fargo (Jeremy Gordon and Mark Bittner), FM MetroCOG (Dan Farnsworth), and Cass County (Jason Benson). 22 members of the public attended the meeting. No special interest groups were represented.


### 2.3 COMMENTS RECEIVED

Four comments were received via email and mailed comment cards after the Public Input Meeting. All comments were related to concerns with access to $52^{\text {nd }}$ Avenue from side streets.

| Topic | Comments | Response |
| :---: | :---: | :---: |
| Access / <br> Roundabouts | I attended public input meeting on Oct. 26th and I have some question (s) remaining. I am concerned about the possibility of constructing a "Roundabout" at the intersection of 63rd St S and 52nd Ave S. Traffic is only going to continue to grow and I believe a "roundabout" would only cause traffic jams. Traffic lights would be a | Various intersection configurations are being considered as part of the planning study. At major intersections like $63^{\text {rd }} \mathrm{St}$ $S$ both roundabouts and traffic signals are being considered. Further analysis, including intersection LOW, will assist in determining the appropriate intersection configuration. |
| Access | Concerned with limiting of movements at $53{ }^{\text {rd }}$ Avenue and $52^{\text {nd }}$ Avenue intersection. <br> See Appendix E for full comment. | At the intersection of $53^{\text {rd }}$ Ave $S$ and $52^{\text {nd }}$ Ave S, five intersection alternatives are being considered with varying access control. A full access, $3 / 4$ access, J-turn, R-cut and right in / right out are currently being considered. A |
| Access | I would like to request an access to my street as you travel from west to east on $52^{\text {nd }}$ Avenue. When I came to the public Input Meeting on Oct $26^{\text {th }}$ I saw the turn possibilities. I am sure the folks living on the street on the southside of $52^{\text {nd }}$ Avenue would also appreciate having access also. The greatest solution would be to have no center | A full access intersection is one of alternate intersection configurations being considered for the McMahon Estates Circle intersection as well as $68^{\text {th }} \mathrm{St} \mathrm{S}$ intersection. |
| Access | Would prefer full access intersection at $53^{\text {rd }}$ Avenue and $52^{\text {nd }}$ Street. See Appendix E for full comment. | At the intersection of $53^{\text {rd }}$ Ave $S$ and $52^{\text {nd }}$ Ave $S$, five intersection alternatives are being considered with varying access control. A full access, $3 / 4$ access, J-turn, R-cut and right in / right |

### 2.4 PUBLIC CONCERNS

There were no controversial issues stated by the public. Concerns about the proposed project were related to the preferred intersection configurations serving commercial and residential developments.

## 3) PUBLIC INPUT MEETING \#2

### 3.1 MEETING DETAILS

Public Input Meeting \#2 was held at the Faith Journey Lutheran Church, $42040^{\text {th }}$ Avenue South, West Fargo, ND, on December 21, 2017 from 5:00 PM to 7:00 PM. The meeting was an open house format, with large format aerial photos of the proposed project corridor and side boards showing proposed intersection alternatives.

The meeting was held to inform the public about alternatives being considered for the upcoming project as well as gather input on existing issues and proposed improvements.

### 3.2 ATTENDEES

The meeting had a total of seven attendees including the project team. The project team was represented by Houston Engineering, Inc. (Jeff Lansink and Adam Ruud), WSB and Associates (Tony Heppelman), The City of Fargo (Jeremy Gordon and Mark Bittner), and FM MetroCOG (Dan Farnsworth). One member of the public attended the meeting. No special interest groups were represented.

## Memorandum

To: $\quad$ 52nd Avenue Study Review Committee
From: Tony Heppelmann, P.E. Adam Smith, AICP

Date: January 9, 2018

Re: $\quad$ Technical Memo No. 3
Identification of Issues and Project Purpose \& Need
WSB Project No. 10745-000

## 1) PROJECT PURPOSE

The purpose of the project is to proactively prepare for long-term growth and development in the project area by addressing future capacity and safety needs and improving 52nd Avenue to serve as an important link in the multimodal transportation system. The project is located along 52nd Avenue between 45th Street and Sheyenne Street in the City of Fargo. See Figure 1 for a map showing the project corridor. Figures $\mathbf{2 a}$ and $\mathbf{2 b}$ show aerial images of the project corridor.

The goals associated with this project are as follows:

- Enable the 52nd Avenue corridor to fulfill its intended transportation role as a minor arterial in the roadway network and its role as an Active Living Street east of Veterans Boulevard consistent with the Go2030 Fargo Comprehensive Plan.
- Provide adequate capacity to carry forecasted 2040 traffic volumes at an acceptable level of service (LOS "D" or better), minimizing travel delays.
- Minimize the potential for crashes in the corridor.
- Provide for safe and comfortable movement of non-motorized transportation users (e.g.
pedestrians and bicyclists) along and across the corridor consistent with Go2030. This includes guidelines associated with the Active Living Streets and City-Wide Trail Loop catalyst projects as well as initiatives focused on bicycle and pedestrian infrastructure, complete streets, and the tree canopy.
- Support orderly and efficient economic development in the project area consistent with local and regional plans including the Go2030 initiative to invest in amenities and beautification as an economic development tool.
- Maintain 52nd Avenue and associated structures in a state of good repair over the life of the project.

The sections that follow describe the existing conditions along the project corridor, the needs for the project, and measurable outcomes associated with the project goals.

## 2) EXISTING CONDITIONS SUMMARY

Within the project corridor, 52nd Avenue is currently a two-lane rural section with left- and rightturn lanes at major intersections. East of 45th Street, 52nd Avenue becomes a 4-lane divided arterial and it becomes a 6-lane section near I-29. The section of 52nd Avenue included in this project is classified as a minor arterial in the approved functional classification plan and the Fargo-Moorhead 2014 Long Range Transportation Plan. In the approved functional classification plan 52nd Avenue becomes a principal arterial east of I-29. Other minor arterials intersecting with 52nd Avenue include 45th Street and 170th Avenue (Sheyenne Street). See Figure 3 for the approved functional classification map. The only existing signalized intersection in the project corridor is at 45th Street and 52nd Avenue. ${ }^{1}$ The intersection of Sheyenne Street and 52 nd Avenue is a roundabout. The other intersections have stop signs on the intersecting side streets.

The posted speed on 52nd Avenue within the study area is 45 mph . As shown in Figure 4, the existing traffic volumes range from 5,845 (average daily traffic) on the west end of the project corridor at Sheyenne Street to 11,840 on the east end of the project corridor, as 52nd Avenue nears the $\mathrm{l}-29$ interchange. Currently heavy commercial traffic is about 4 percent of the total traffic on 52nd Avenue.

The corridor has a wide right-of-way and access is primarily limited to other public streets. North of 52nd Avenue within the project corridor, land uses are primarily residential and commercial. To the south of 52nd Avenue, there are currently commercial and undeveloped agricultural land uses, with the Deer Creek residential neighborhood located further to the south along 63rd Street. The undeveloped portions of the 52nd Avenue corridor are guided for a future land use mix of primarily commercial and residential (low to high density) uses based on current land use plans.

The only existing dedicated bicycle/pedestrian facilities along the corridor are a section of multiuse trail along the north side of 52nd Avenue between 47th Street and 45th Street, and marked crosswalks and pedestrian ramps at the intersections of 52nd Avenue with 45th Street and Sheyenne Street. Several side streets within the corridor have sidewalks that terminate at or near 52nd Avenue. No fixed-route transit services are currently provided along the project corridor.

The project corridor includes two bridges: Bridge 09-139-30.0 over the Sheyenne River at the west end of the corridor, and Bridge 09-140-30.0 over County Drain 27 west of 45th Street. Bridge 09-139-30.0 over the Sheyenne River is a three-span prestressed concrete bridge with two travel lanes and a deck width of 33.1 feet. Bridge 09-140-30.0 over Drain 27 is a single span prestressed concrete bridge with two travel lanes and a deck width of 39.4 feet.

Go2030, the Fargo Comprehensive Plan, designates a number of "catalyst projects" with associated initiatives and design recommendations. Two of these projects are located partially within the project corridor. First, the segment of 52nd Avenue between Veterans Boulevard and 45th Street is designated as an Active Living Street, which is described in Go2030 as a key corridor within the city with the potential to support multiple modes of transportation, incorporate green stormwater infrastructure, and become a great public space with attractive streetscapes. Additionally, the alignment of County Drain 27, which crosses 52nd Avenue, is identified as part of the All Season City-Wide Trail Loop in the comprehensive plan, which is intended to connect neighborhoods and walkable mixed use centers for users who are walking, biking, and potentially cross-country skiing. These projects are identified in Figure 1.

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## 3) NEEDS FOR THE PROJECT

There are six needs for the 52nd Avenue project: system linkages, capacity, long-term safety, non-motorized transportation, economic development, and long-term pavement and structure conditions.

### 3.1 SYSTEM LINKAGES

As the project area develops and other planned transportation improvements are made based on the Fargo-Moorhead Long Range Transportation Plan (including, for example, the extension of 45th Street to 64th Avenue), 52nd Avenue will gain a more important role in the transportation network. 52nd Avenue is classified as a minor arterial within the project corridor and becomes a principal arterial east of I-29, which has an interstate highway classification. It is approximately two miles to the north to 32nd Avenue, which is a parallel east-west arterial with access to l-29 and it is approximately 4 miles to the south to a parallel roadway with access to I-29 (Cass County Road 14). The primary purpose of a minor arterial is to provide mobility for medium to longer trips within the region.

The planned future construction of collector and local roadways to the south and west of the 52nd Avenue corridor will provide connections to 52nd Avenue increasing the utility of the corridor as an east-west arterial providing access to I-29. The current design of 52nd Avenue as an undivided two-lane roadway is inconsistent with its role in the transportation network as an arterial roadway intended to provide safe mobility for medium and longer trips within the region.

The segment of 52nd Avenue between Veterans Boulevard and 45th Street is also designated as part of a network of Active Living Streets in the City of Fargo's Go2030 plan, which includes recommendations to include features that support all modes of transportation, including non-motorized facilities, transit, streetscape elements and other amenities such as natural stormwater features. ${ }^{2}$ The existing corridor currently lacks many of these features.

52nd Avenue is also recognized as a prominent regional facility in the Fargo-Moorhead 2040 Long Range Transportation Plan, Metro 2040. Between Sheyenne Street and 45th Street, 52nd Avenue is identified as a truck route in the regional freight network. Between Veterans Boulevard and 45th Street, 52nd Avenue is identified as a route for potential future transit coverage by 2040. Additionally, the entire project corridor is identified as part of the Regionally Significant Transportation Infrastructure (RSTI) network. The RSTI is comprised of regional arterial roadways that are highly contiguous across multiple jurisdictions and intended to operate efficiently on a day-to-day basis but could also serve as emergency detours or evacuation routes during disasters (e.g. flood events).

A goal of this project is to improve 52nd Avenue consistent with the role as a minor arterial in the roadway network and its role as an Active Living Street east of Veterans Boulevard.

Measurable outcomes associated with this need include the following:

- The facility accommodates medium-to-long trips (a majority of trips are longer than five miles)
- Design speed is 40 to 45 miles per hour
- Access density is minimized to the extent feasible
- Turn lanes and/or medians are provided to the extent feasible
- Design enables addition of future transit service
- Linkages are provided for non-motorized transportation users


### 3.2 CAPACITY

Another goal for this project is to provide adequate capacity to carry the 2040 forecast traffic at an acceptable level of service (LOS "D" or better). ${ }^{3}$ A simulation of existing traffic conditions was developed using SimTraffic. Under existing conditions (2017 traffic volumes and traffic control), 52nd Avenue is generally providing a high level of service; however, there are fairly long delays at some side streets with unsignalized intersections, including the northbound approach at 63rd Street, the northbound approach at Veterans Boulevard, and the southbound approach at 53rd Avenue. Each of these intersections had at least one movement at LOS "F" in either the AM or PM peak hour. Tables 1 and 2 summarize the results of this analysis for the AM and PM peak hours for 2017 conditions.

[^1]Table 1: Existing 2017 Measures of Effectiveness (MOE) - AM Peak Hour

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic
${ }^{3}$ Level of Service (LOS) is a measure of traffic operations ranging from LOS " $A$ " to LOS " $F$," where LOS "A" represents free-flowing traffic and LOS "F" represents consistent, substantial delays.

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Table 2: Existing 2017 Measures of Effectiveness (MOE) - PM Peak Hour


2040 Travel Demand Forecasts were developed for the corridor using the Fargo-Moorhead Council of Governments (FM COG) Travel Demand Model and household and employment forecasts. The 2040 Forecast ADTs are shown on Figure 5. The 2040 ADT forecasts show an increase of 6,755 vehicles per day on the west end of the corridor and 8,760 vehicles per day on the east end of the corridor.

A simulation of the future traffic conditions with no improvements to 52 nd Avenue was developed using SimTraffic (including no intersection improvements). Based on modeling of traffic operations under the future no build scenario, vehicle delays are anticipated to increase substantially, with many additional approaches with delays at LOS "F." Tables 3 and 4 summarize the results of this analysis for the future no build AM and PM peak hours. Below is a discussion of the operations expected at each intersection moving from west to east.

- Sheyenne Street and 52nd Avenue - This roundabout is forecast to operate at LOS "F" in the am peak hour and LOS " $E$ " in the pm peak hour. The worst approaches are the northbound approach in the am peak hour and the southbound and westbound approaches in the pm peak hour.
- 63rd Street and 52nd Avenue - The northbound and southbound approaches of this intersection are forecast to operate at LOS "F" in the am and pm peak hour.
- 53rd Avenue and 52nd Avenue - This intersection is forecast to operate at an acceptable LOS "D" or better in the am and pm peak hours.
- Veteran's Boulevard and 52nd Avenue - The northbound and southbound approaches of this intersection are forecast to operate at LOS " $F$ " in the am and pm peak hour.

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- 53rd Street and 52nd Avenue - The northbound and southbound approaches of this intersection are forecast to operate at LOS "F" in the am and pm peak hour.
- 47th Street and 52nd Avenue - The southbound approach of this intersection is forecast to operate at LOS " F " in the am and pm peak hours. There is currently no south leg on this intersection, but it is planned to accommodate future development.
- 45th Street and 52nd Avenue - This intersection is currently signalized and is forecast to operate at LOS "D" in the am and pm peak hours.

Table 3: 2040 No-Build Measures of Effectiveness (MOE) - AM Peak Hour

| Intersection |  | $\underset{\mathbf{r}}{\mathrm{App}}$ | Total Delay by Movement |  |  | LOS by Moveme nt |  |  | LOS by Approac h (Sec/Ve h) |  | LOS by Intersecti on (Sec/Veh) |  | Appr | Average \& Maximum Traffic Queueing (feet) |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 은 | Location |  |  |  |  |  | Left- | Turn |  |  |  | Thro |  | ugh |  | Right | -Turn |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | $\begin{gathered} \text { Dela } \\ \mathbf{y} \end{gathered}$ | LOS | $\begin{gathered} \hline \text { Ave } \\ \text { Queu } \\ \mathrm{e} \end{gathered}$ | $\begin{gathered} \text { Max } \\ \text { Queu } \\ \text { e } \end{gathered}$ | $\begin{array}{\|c} \text { Storag } \\ \mathrm{e} \end{array}$ | $\begin{gathered} \hline \text { Ave } \\ \text { Queu } \\ \text { e } \end{gathered}$ | $\begin{array}{\|c} \hline \text { Max } \\ \text { Queu } \\ \text { e } \end{array}$ | $\begin{gathered} \text { Storag } \\ \mathrm{e} \end{gathered}$ | $\begin{gathered} \text { Ave } \\ \text { Queu } \\ \text { e } \end{gathered}$ | $\begin{array}{r} \text { Max } \\ \text { Queu } \end{array}$ $\mathrm{e}$ | Storag |
|  | 1: Sheyenne St \& 52nd Avenue S (Based on HCM) | NB | N/A |  |  |  |  |  | 245 | F | 114 | F |  | NB |  |  |  |  | 1281 |  |  |  |  |
| $\stackrel{\rightharpoonup}{\mathbf{0}} \mathbf{0}$ |  | WB |  |  |  |  |  |  | 23 | C |  |  | WB |  |  |  |  | 158 |  |  |  |  |
| $\begin{gathered} \mathbf{0} \\ \mathbf{0} \end{gathered}$ |  | SB |  |  |  |  |  |  | 14 | B |  |  | SB |  |  |  |  | 124 |  |  |  |  |
|  |  | EB |  |  |  |  |  |  | 14 | B |  |  | EB |  |  |  |  | 73 |  |  |  |  |
| $\frac{\circ}{\mathbf{\circ}}$ | 2: 63rd St \& 52nd Avenue S | NB | 698 | 639 | 698 | F | F | F | 696 | F |  |  | NB |  |  |  | 1588 | 1668 |  |  |  |  |
|  |  | WB | 9 | 1 | 0 | A | A | A | 3 | A |  |  | WB | 52 | 124 | 275 |  |  |  |  | 4 | 275 |
|  |  | SB | 53 | 24 | 6 | F | C | A | 27 | D |  |  | SB |  |  |  | 32 | 79 |  | 19 | 55 | 165 |
|  |  | EB | 6 | 7 | 4 | A | A | A | 7 | A |  |  | EB | 14 | 43 | 300 |  | 5 |  | 1 | 10 | 300 |
|  | 3: 53rd Ave S/New 53rd Ext \& 52nd Avenue S | NB | 20 | 13 | 6 | C | B | A | 7 | A |  |  | NB |  |  |  | 22 | 49 |  |  |  |  |
|  |  | WB | 5 | 2 | 1 | A | A | A | 2 | A |  |  | WB | 17 | 47 | 200 |  |  |  |  |  |  |
|  |  | SB | 19 | 16 | 7 | C | C | A | 14 | B |  |  | SB |  |  |  | 21 | 56 |  |  |  |  |
|  |  | EB | 3 | 2 | 0 | A | A | A | 2 | A |  |  | EB | 12 | 36 | 250 |  |  |  |  |  |  |
|  | 4: 53rd Ave S/Veterans Blvd \& 52nd Avenue S | NB | 16 | 0 | 8 | C | A | A | 8 | A |  |  | NB |  |  |  | 4 | 25 |  |  |  |  |
|  |  | WB | 5 | 3 | 2 | A | A | A | 3 | A |  |  | WB | 7 | 32 | 300 |  | 4 |  | 2 | 24 | 300 |
|  |  | SB | 57 | 26 | 7 | F | D | A | 26 | D |  |  | SB | 57 | 158 | 200 | 5 | 28 |  | 34 | 89 | 200 |
|  |  | EB | 7 | 2 | 1 | A | A | A | 3 | A |  |  | EB | 36 | 97 | 275 |  |  |  |  |  |  |
| $\left\lvert\, \begin{gathered} \stackrel{2}{\mathbf{0}} \\ \stackrel{\rightharpoonup}{9} \\ \stackrel{\rightharpoonup}{5} \\ \stackrel{y}{1} \end{gathered}\right.$ | 5: 53rd St/51st Ave \& 52nd Avenue S | NB | 42 | 53 | 25 | E | F | D | 32 | D |  |  | NB |  |  |  | 33 | 102 |  |  |  |  |
|  |  | WB | 8 | 3 | 2 | A | A | A | 3 | A |  |  | WB | 17 | 44 | 150 |  |  |  |  |  |  |
|  |  | SB | 314 | 314 | 310 | F | F | F | 314 | F |  |  | SB |  |  |  | 325 | 490 |  |  |  |  |
|  |  | EB | 6 | 3 | 1 | A | A | A | 3 | A |  |  | EB | 5 | 28 | 175 |  | 3 |  |  | 7 | 175 |
| $\begin{aligned} & \stackrel{\circ}{0} \\ & \stackrel{\rightharpoonup}{0} \\ & \underset{\sim}{2} \\ & \stackrel{\rightharpoonup}{1} \end{aligned}$ | 6: 52nd Avenue S \& 47th St | NB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 6 | 5 | A | A | A | 6 | A |  |  | WB |  |  |  |  |  |  |  |  |  |
|  |  | SB | 87 | 0 | 72 | F | A | F | 79 | F |  |  | SB | 121 | 322 |  |  |  |  |  |  |  |
|  |  | EB | 10 | 4 | 0 | B | A | A | 4 | A |  |  | EB | 14 | 46 | 125 |  |  |  |  |  |  |
|  | 7: 45th St \& 52nd Avenue S | NB | 34 | 36 | 21 | C | D | C | 30 | C | 36 | D | NB | 89 | 177 | 300 | 164 | 262 |  | 130 | 267 | 300 |
|  |  | WB | 60 | 80 | 43 | E | F | D | 63 | E |  |  | WB | 95 | 350 | 250 | 464 | 1021 |  | 215 | 394 | 250 |
|  |  | SB | 44 | 26 | 11 | D | C | B | 32 | C |  |  | SB | 124 | 230 | 500 | 110 | 192 |  | 33 | 87 |  |
|  |  | EB | 29 | 22 | 17 | C | C | B | 24 | C |  |  | EB | 74 | 130 | 250 | 131 | 204 |  |  |  |  |

*Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

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Table 4: 2040 No-Build Measures of Effectiveness (MOE) - PM Peak Hour

| 은 | Intersection | Appr | Total Delay by Movement |  |  | LOS by Movemen t |  |  | LOS by Approac h (Sec/Veh ) |  | LOS by <br> Intersectio <br> $\mathrm{n}(\mathrm{Sec} /$ Veh $)$ |  | Appr | $\begin{gathered} \text { Average } \\ \& \end{gathered}$ |  |  | Maximum Traffic Queu |  |  | eing (feet) |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Location |  |  |  |  | Left-Turn | Through |  |  | Right-Turn |  |  |  |
|  |  |  | L | T | R |  |  |  | L | T |  |  | R | Delay | LOS | Delay | LOS |  | $\begin{gathered} \text { Ave } \\ \text { Queu } \\ \text { e } \end{gathered}$ | $\begin{gathered} \text { Max } \\ \text { Queu } \\ \mathrm{e} \end{gathered}$ | Storage | $\begin{gathered} \text { Ave } \\ \text { Queu } \\ \text { e } \end{gathered}$ | $\begin{gathered} \text { Max } \\ \text { Queu } \\ \text { e } \end{gathered}$ | Storage | $\begin{gathered} \text { Ave } \\ \text { Queu } \\ \text { e } \end{gathered}$ | Max Queu e | Storage |
|  | 1: Sheyenne St \& 52nd Avenue $S$ (Based on HCM) | NB | N/A |  |  |  |  |  |  |  | 11 | B | 49 | E | NB |  |  |  |  | 83 |  |  |  |  |
| $\left\lvert\, \begin{aligned} & \mathbf{3} \\ & \mathbf{o l} \end{aligned}\right.$ |  | WB |  |  |  |  |  |  | 35 | E | WB |  |  |  |  |  |  | 364 |  |  |  |  |
| $\left\|\begin{array}{l} \mathbf{y} \\ \mathbf{0} \\ \boldsymbol{\sim} \end{array}\right\|$ |  | SB |  |  |  |  |  |  | 100 | F | SB |  |  |  |  |  |  | 577 |  |  |  |  |
|  |  | EB |  |  |  |  |  |  | 17 | C | EB |  |  |  |  |  |  | 53 |  |  |  |  |
|  | 2: 63rd St \& 52nd Avenue S | NB | 168 | 177 | 136 | F | F | F | 140 | F |  |  | NB |  |  |  | 319 | 693 |  |  |  |  |
|  |  | WB | 7 | 1 | 1 | A | A | A | 3 | A |  |  | WB | 66 | 136 | 275 |  | 9 |  |  | 4 | 275 |
|  |  | SB | 88 | 59 | 6 | F | F | A | 46 | E |  |  | SB |  |  |  | 42 | 118 |  | 17 | 51 | 165 |
|  |  | EB | 5 | 5 | 2 | A | A | A | 5 | A |  |  | EB | 14 | 47 | 300 |  |  |  | 1 | 19 | 300 |
| $\begin{array}{\|c} \stackrel{\circ}{0} \\ \stackrel{\rightharpoonup}{\omega} \\ \stackrel{\rightharpoonup}{2} \\ \stackrel{\rightharpoonup}{1} \end{array}$ | 3: 53rd Ave S/New 53rd Ext \& 52nd Avenue S | NB | 28 | 19 | 6 | D | C | A | 8 | A |  |  | NB |  |  |  | 26 | 69 |  |  |  |  |
|  |  | WB | 4 | 3 | 1 | A | A | A | 3 | A |  |  | WB | 6 | 32 | 200 |  |  |  |  |  |  |
|  |  | SB | 22 | 32 | 11 | C | D | B | 18 | C |  |  | SB |  |  |  | 14 | 49 |  |  |  |  |
|  |  | EB | 6 | 1 | 0 | A | A | A | 1 | A |  |  | EB | 11 | 42 | 250 |  |  |  |  |  |  |
|  | 4: 53rd Ave S/Veterans Blvd \& 52nd Avenue S | NB | 98 | 74 | 21 | F | F | C | 57 | F |  |  | NB |  |  |  | 13 | 58 |  |  |  |  |
|  |  | WB | 5 | 4 | 2 | A | A | A | 4 | A |  |  | WB | 3 | 32 | 300 |  |  |  | 3 | 31 | 300 |
|  |  | SB | 1123 | 765 | 782 | F | F | F | 906 | F |  |  | SB | 293 | 300 | 200 | 1621 | 2058 |  | 92 | 274 | 200 |
|  |  | EB | 20 | 2 | 1 | C | A | A | 6 | A |  |  | EB | 53 | 136 | 275 |  |  |  |  |  |  |
| $\begin{aligned} & \circ \\ & \stackrel{\circ}{0} \\ & \stackrel{2}{0} \\ & \stackrel{2}{2} \\ & \stackrel{1}{1} \end{aligned}$ | 5: 53rd St/51st Ave \& 52nd Avenue S | NB | 39 | 66 | 10 | E | F | B | 15 | C |  |  | NB |  |  |  | 27 | 70 |  |  |  |  |
|  |  | WB | 5 | 4 | 4 | A | A | A | 4 | A |  |  | WB | 8 | 32 | 150 |  |  |  |  |  |  |
|  |  | SB | 505 | 448 | 542 | F | F | F | 503 | F |  |  | SB |  |  |  | 479 | 555 |  |  |  |  |
|  |  | EB | 11 | 2 | 1 | B | A | A | 2 | A |  |  | EB | 6 | 27 | 175 |  |  |  |  |  |  |
|  | 6: 52nd Avenue S \& 47th St S | NB | 0 | 0 | 0 | A | A | A | 0 | A |  |  | NB |  |  |  |  |  |  |  |  |  |
|  |  | WB | 0 | 7 | 6 | A | A | A | 7 | A |  |  | WB |  |  |  |  |  |  |  | 11 | 175 |
|  |  | SB | 105 | 0 | 80 | F | A | F | 91 | F |  |  | SB | 77 | 186 |  |  |  |  |  |  |  |
|  |  | EB | 21 | 3 | 0 | C | A | A | 4 | A |  |  | EB | 15 | 54 | 125 |  |  |  |  |  |  |
|  | 7: 45th St \& 52nd Avenue S | NB | 39 | 39 | 11 | D | D | B | 29 | C | 35 | D | NB | 72 | 149 | 300 | 128 | 200 |  | 68 | 134 | 300 |
| $\underset{N}{0}$ |  | WB | 45 | 53 | 29 | D | D | C | 46 | D |  |  | WB | 164 | 350 | 250 | 408 | 1124 |  | 126 | 394 | 250 |
| $\begin{array}{\|c} \stackrel{5}{6} \\ \overline{6} \end{array}$ |  | SB | 34 | 33 | 23 | C | C | C | 31 | C |  |  | SB | 95 | 155 | 500 | 131 | 217 |  | 95 | 210 |  |
|  |  | EB | 28 | 27 | 18 | C | C | B | 24 | C |  |  | EB | 23 | 65 | 250 | 125 | 219 |  |  |  |  |

${ }^{*}$ Roundabout MOE based on HCM $6^{\text {th }}$ Edition. Roundabout Queues are $95^{\text {th }}$ percentile. All other MOEs from SimTraffic

A goal of this project is to provide adequate capacity for the forecasted 2040 traffic volumes to maintain an acceptable level of service in the corridor (including the crossings of the Sheyenne River and County Drain 27). The measurable outcome associated with this goal is that all intersections in the corridor operate at LOS " D " or better and no individual movement is worse than LOS "D" and that roadway capacity provided at bridge crossings is consistent with the remainder of the corridor.

### 3.3 SAFETY

Over a five-year period between 2012 and 2017, 41 crashes have occurred in the corridor, including 26 at three of the intersections within the corridor (Sheyenne Street, Veterans Boulevard, and 45th Street). Two-thirds of the crashes were property damage only and the remaining crashes involved minor injuries, and overall, the crash rate on 52nd Avenue is relatively low. However, based on the projected growth in traffic volumes and the presence of five unsignalized intersections along the project corridor, there are concerns that, as delays on the side streets increase with growing traffic volumes, motorists will take more chances when turning onto 52nd Avenue or traveling across 52nd Avenue at these locations.

A goal of this project is to minimize the potential for crashes in the corridor following project completion. The measurable outcome associated with this goal is that the five-year average crash rate for segments and intersections should not exceed the statewide averages for similar facilities (the actual rate may vary by alternative facilities).

## Non-motorized Transportation

There are limited bicycle/pedestrian facilities along the corridor under existing conditions (see Figures 2a and 2b). As the project corridor continues to develop, demand for safe nonmotorized access along and across 52nd Avenue will increase. Osgood Park is located north of 52nd Avenue and includes an existing shared-use path that leads to 52nd Avenue, terminating just north of the roadway. Legacy Elementary School is located less than one mile west of the project corridor. Several side streets within the corridor have sidewalks that terminate at or near 52nd Avenue, including 63rd Street, Veterans Boulevard, 51st Avenue, and 47th Street.

The Go2030 plan includes Active Living Streets and City-Wide Trail Loop catalyst projects as well as initiatives focused on bicycle and pedestrian infrastructure, complete streets, and the tree canopy. The segment of 52nd Avenue between Veterans Boulevard and 45th Street is designated as part of a network of Active Living Streets, and the City-Wide Trail Loop follows an alignment that crosses the 52nd Avenue corridor along County Drain 27. The facilities and amenities associated with these initiatives are currently absent from most of the project corridor.

A goal associated with this project is to provide for the safe and comfortable movement of non-motorized transportation users along and across the corridor consistent with Go2030 catalyst projects and initiatives. The measurable outcomes associated with this need include the presence of bicycle and pedestrian facilities on both north and south sides of 52 nd Avenue, multiple crossings so that pedestrians do not need to walk more than 0.5 mile to cross 52nd Avenue (including a crossing for the City-Wide Trail Loop and consideration of grade-separated crossings), crossing distances and vehicle speeds that are minimized to the extent feasible, and incorporation of landscaping and vegetation that contribute to pedestrian/bicyclist safety and comfort.

## Economic Development

The southwestern portion of Fargo is experiencing rapid development along I-29, including the 52nd Avenue corridor. As a result, the city anticipates increases in traffic volumes on 52nd Avenue due to new development south and west of the project corridor. Current land use plans guide the undeveloped area south of the project corridor for commercial uses and a mix of lower to higher density residential uses.

The Go2030 plan includes an initiative focusing on the use of amenities and beautification as an economic development tool. Recommendations associated with this initiative include prioritizing pedestrian and bicycle streetscape improvements when upgrading infrastructure and investing in trails, parks, and other public spaces. These features are currently limited along the project corridor.

Additionally, the Fargo Moorhead Long Range Transportation Plan (LRTP) identifies additional future transportation improvements in the project area. As part of these assumed improvements,
changes in the transportation network include the proposed widening of Sheyenne Street and the proposed extension of 45th Street south of 52nd Avenue. These improvements, along with the concurrent development that is anticipated, will have a substantial impact on the traffic demand along the 52nd Avenue corridor.

A goal of this project is to support orderly and efficient economic development in the project area consistent with local and regional plans, including the Go2030 initiative to invest in amenities and beautification as an economic development tool. The measurable outcomes associated with this need include accommodating access from the planned roadway improvements that will serve future development along 52nd Avenue (including infill development) and the inclusion of streetscape and/or public realm design characteristics that will help attract businesses and residents to the area.

## Long-term Pavement and Structure Condition

Overall, the existing pavement along 52nd Avenue is in relatively good condition. The International Roughness Index (IRI), which serves as a proxy for ride quality, ranges from 88.7 inches per mile ( $\mathrm{in} / \mathrm{mi}$ ) to $175.5 \mathrm{in} / \mathrm{mi}$ within the project corridor. A "Good" IRI value is less than $95 \mathrm{in} / \mathrm{mi}$, while an IRI of less than $170 \mathrm{in} / \mathrm{mi}$ is considered "Acceptable." Therefore, there is at least one segment of the project corridor with an IRI higher than what would be considered acceptable (between 45th Street and 44th Street), and, as pavement condition deteriorates in the future, other segments will also approach an unacceptable level.

The Ride Quality Index (RQI), which is an alternative measure of pavement roughness, quantifying what the typical motorist feels when driving on a roadway, ranges from 72.32 to 85.99 within the project corridor (out of 100). The Pavement Quality Index (PQI), which is used to quantify the overall condition of pavement (including the level of pavement distress and visible defects such as cracks, patches, and ruts), ranges from 78.6 to 92.99 on 52nd Avenue (out of 100). These ratings indicate that the current pavement condition ranges from "satisfactory" to "good."

Neither of the bridges along the project corridor is deficient. Bridge 09-139-30.0 over the Sheyenne River has an overall sufficiency rating of 82.20. Bridge 09-140-30.0 over Drain 27 has an overall sufficiency rating of 100 and has a skew of 20 degrees.

As project alternatives are considered, asset management considerations should account for the current and long-term future pavement and bridge conditions along 52nd Avenue. Allowing IRI, RQI, PQI to deteriorate too low will make the ride for motorists unpleasant, potentially limiting mobility and creating vehicle damage. Allowing bridge conditions to deteriorate over the long-term could also result in an unpleasant riding experience along with potential safety concerns.

A goal of this project is to maintain 52nd Avenue and associated structures in a state of good repair over the life of the project. The measurable outcome associated with this need includes maintaining a "good" PQI (above 60) for all segments throughout the life of the project and ensuring that the bridges maintain a sufficient rating throughout the life of the project.







| Topic | Comments | Response |
| :---: | :---: | :---: |
| Pedestrian Crossing | Consider a pedestrian underpass near the proposed school site for the safety of children trying to cross $52^{\text {nd }}$ Ave from North \& South from Rocking Horse and McMahon Estates. | Creating a corridor meeting the needs for all forms of transportation is a key project objective. Feasibility and location of pedestrian underpasses will be considered in the environmental document. |
| Access | Concerned with restricting northbound to westbound turn movements from $53^{\text {rd }}$ Avenue. <br> See Appendix J for full comment. | At the intersection of $53^{\text {rd }}$ Ave $S$ and $52^{\text {nd }}$ Ave $S$, five intersection alternatives are being considered with varying access control. A full access, $3 / 4$ access, J-turn, R-cut and right in / right out are currently being considered. A project is also planned for 2018 to pave $53^{\text {rd }}$ Ave S. |
| Access | We are concerned about the ability to access $52^{\text {nd }}$ Ave from our location to go both east and west with truck traffic. Most of our traffic is large semi traffic. We are requesting a full access intersection at 52 Ave / 53 Ave. Please put us on record that we are opposed to all options except the full access option and 52nd and 53 ${ }^{\text {rd }}$ Aves. | At the intersection of $53^{\text {rd }}$ Ave $S$ and $52^{\text {nd }}$ Ave $S$, five intersection alternatives are being considered with varying access control. A full access, $3 / 4$ access, J-turn, R-cut and right in / right out are currently being considered. A project is also planned for 2018 to pave $53{ }^{\text {rd }}$ Ave S. |
| See Appendix J for full comment. |  |  |

## 4) PUBLIC CONCERNS

There were no controversial issues stated by the public. Concerns about the proposed project were related to grade separated pedestrian crossings of $52^{\text {nd }}$ Ave $S$ and preferred intersection configurations serving a commercial area.

## D TECHNICAL MEMORANDUM 4 - EVALUATION OF PROJECT ALTERNATIVES

# Technical Memorandum 4 - Evaluation of Project Alternatives 

Subject: Technical Memorandum 4 - Evaluation of Project Alternatives
Date: February 14, 2018
Project: FM MetroCOG Project 2017-0057
City of Fargo Project MS-17-A0
NDDOT Project SU-8-984(164), PCN 22007
HEI Project 6631-006

## 1) INTRODUCTION

The purpose of the $52^{\text {nd }}$ Ave S Study is to determine a proactive plan for the improvements needed for $52^{\text {nd }}$ Ave $S$ between Sheyenne Street in the City of West Fargo and $45^{\text {th }}$ St $S$ in the City of Fargo to serve as an important link in the multimodal transportation system. Thereby, accommodating the long-term growth and development in the project area by addressing future capacity and safety needs.

The corridor segment under study is currently a two-lane rural section with left and right turn lanes at major intersections. East of $45^{\text {th }}$ St S, $52^{\text {nd }}$ Ave $S$ becomes a four-lane divided arterial with the potential for dual left turn lanes and right turn lanes at the major intersections. $52^{\text {nd }}$ Ave $S$ becomes a 6-lane section near Interstate 29.

This memo discusses the evaluation of project alternatives related to:

- Geometrics
- Traffic Operations/Access Control
- Structural Requirements
- Utility Impacts
- Pedestrian Facilities
- Right of Way
- Environmental Impacts
- Anticipated Construction Costs


## 2) ANALYSIS

### 2.1 ALTERNATIVE A - NO-BUILD

The No-Build Alternative would leave the corridor in its current condition with no improvements. This Alternative does not address the existing deficiencies within the project corridor, including poor intersection level of service (LOS) and deteriorating pavement conditions. Traffic volumes are expected to nearly double by the year 2040, resulting in decreased LOS throughout the corridor, as well as continued maintenance measures to maintain the functionality of the roadway.

This Alternative would maintain the existing geometrics of the two-lane rural corridor with left and right turn lanes at the major intersections. Existing pedestrian facilities would continue to terminate at the corridor right of way limits, prohibiting connectivity and reducing functionality of the trail systems. As no modifications would be made to the project corridor, the existing structures would remain in place but would require replacement in the future as they continue to deteriorate and become structurally deficient. This option will have no utility, right-of- way or environmental impacts. The probable cost to construct Alternative A is $\$ 0$.

### 2.2 ALTERNATIVE B - 4 LANE URBANSECTION

The Build Alternative would reconstruct $52^{\text {nd }}$ Ave S from Sheyenne St east to $45^{\text {th }}$ St S to a four lane, urban section, accommodating long term growth and development in the project area. The improved corridor would meet a design speed of 45 mph and increase traffic capacity while providing pedestrian facility connectivity to promote an active, living corridor. The base alternative consists of a combination of signalized and full access intersections for comparison purposes. The final Build Alternative may be a combination of the additional intersection configurations presented following the base option discussion.

## Roadway Geometrics

$52^{\text {nd }}$ Ave $S$ would be realigned to be centered within the existing right of way. Lane transitions will be made over a distance determined by multiplying the design speed ( 45 mph ) and the width of the transition meeting current AASHTO standards. The typical section would consist of two 11' wide eastbound thru lanes and two 11' wide westbound thru lanes. The eastbound and westbound roadways would be separated by a raised median that varies in width from approximately 4' (where a left turn lane is present) up to 15 '. See Figure 1 for a typical layout. See Figure 2 for the proposed typical section.

Figure 1 - Full Access Intersection Layout


Figure 2 - Full Access Intersection Typical Section


PROPOSED TYPICAL - FULL ACCESS

A 10' wide shared use path would be constructed along the north and south right-of-way limits separated from $52^{\text {nd }}$ Ave S by a grass boulevard varying in width from approximately 8 up to 35 . To improve intersection pedestrian crossing safety and vehicular sight distances, the boulevard width
will be reduced to a minimum 6 ' wide grass boulevard immediately adjacent to all intersections within the corridor.

Right and left turn lanes would also be implemented at the following locations along the eastbound and westbound roadways.

- $68^{\text {th }}$ St S
- Eastbound Right Turn Lane
- Westbound Left Turn Lane
- $63^{\text {rd }} \mathrm{StS}$
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane
- 53 rd Ave S
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane
- Veterans Blvd
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane
- $51^{\text {st }}$ Ave $S$
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane
- $47^{\text {th }}$ St S
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane
- $45^{\text {th }}$ St S
- Eastbound Right Turn Lane
- Eastbound Left Turn Lane
- Westbound Right Turn Lane
- Westbound Left Turn Lane

Turn lane lengths will be designed to meet NDDOT Standards for Traffic Operations as well as deceleration and queue lengths.

Due to the roadway widening and shared use paths, it is anticipated the existing roadway ditches will be eliminated. To maintain local drainage, it is anticipated that $52^{\text {nd }}$ Ave $S$ will be lowered to the

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extent possible to allow drainage within the right of way to drain toward the roadway. Snow drifting will also be considered when determining the elevation of the roadway. To prevent drifting, portions of the roadway will likely need to be set higher and inlets installed within the boulevard to maintain drainage.

During detailed design, an option utilizing a wider median with narrower boulevards may be considered. However, this will result in an increased pavement width and project cost to provide a maximum of 6' negative offset between left turn lanes.

The probable cost to construct Alternative $B$ is $\$ 20,780,000$.

## 3) TRAFFIC OPERATIONS/ACCESS CONTROL

Corridor travel times for Alternative B, utilizing signalized and full access intersections, are expected to be approximately 280.6 seconds with an average speed of 31 mph . With 2040 traffic projections, this Alternative results in the following intersection LOS. The LOS shown for the signalized and roundabout intersections (Sheyenne St, 63 rd Street and Veterans Blvd) are for the overall intersection. The LOS shown for the approach road stop condition ( $53{ }^{\text {rd }}$ Avenue, $51^{\text {st }}$ Avenue and $47^{\text {th }}$ Street) is the worst leg of the intersection:

- Sheyenne St: A (am) A (pm)
- 63 ${ }^{\text {rd }}$ Street: $\quad B(a m) \quad B(p m)$
- $53^{\text {rd }}$ Avenue: D (am, SB Leg) E (pm, NB Leg)
- Veterans Blvd: A (am) B (pm)
- 51st Avenue: F (am, SB Leg) $\mathrm{F}(\mathrm{pm}$, SB Leg)
- $47^{\text {th }}$ Street: $\quad F(a m, S B$ Leg) $D(p m$, SB Leg $)$
- $45^{\text {th }}$ Street: $\quad \mathrm{C}(\mathrm{am}) \quad \mathrm{C}(\mathrm{pm})$

Without modifications to the roundabout at the Sheyenne Street and $52^{\text {nd }}$ Avenue S intersection, the roundabout is forecasted to operate at LOS F and LOS E in the am and pm peak hours in 2040. Currently the roundabout operates at a LOS D. It is projected the roundabout will operate at a LOS F by the year 2030 if no improvements are made. Therefore, recommended improvements include making the northbound and southbound legs of the roundabout 2-lane approaches with two lanes through the roundabout in the northbound and southbound directions. The other improvement would be an additional right only lane on the east approach of the roundabout.

## 4) STRUCTURAL REQUIREMENTS

Due to the reconstruction of $52^{\text {nd }}$ Ave $S$ from a two-lane rural section to a four-lane urban section, the existing structures crossing the Sheyenne River and Drain 27 will be impacted. The existing Sheyenne River crossing is a 110', three span, prestressed concrete structure, with a clear width of
30.8'. The sufficiency rating of the structure was 82.20 in 2016. The south side of the bridge substructure also functions as a flood control structure equipped with two $4^{\prime} \times 16$ ' gates and a weir wall.

In order, to accommodate the four-lane section and 10 ' wide shared use path north and south of $52^{\text {nd }}$ Ave S , the bridge will need to be widened to approximately $90^{\prime}$. Alternatives are being considered to widen the existing structure in place to avoid impacts to the existing control structure or to reconstruct the existing bridge and modify the control structure as needed. In addition, an option to widen the bridge to accommodate the four-lane section with separate, adjacent pedestrian facilities is being considered.

The existing Drain 27 crossing is a 66', single span, prestressed concrete structure with a clear width of $36.7^{\prime}$. The sufficiency rating of the structure in 2016 was 100.00 . Similar to the Sheyenne River crossing, the Drain 27 crossing will require approximately a 90 ' wide deck to accommodate the four-lane section and improved pedestrian facilities. Due to the width requirement and the desire to provide a pedestrian crossing underneath the bridge, a full reconstruct of the bridge is recommended. The structure is anticipated to be approximately 140 ' long and 90 ' wide. To reduce project costs due to the extra deck width resulting from the median, an additional option constructing two separate structures for eastbound and westbound traffic is being considered. The structure is anticipated to be a 3-span structure with piers oriented to allow for a pedestrian grade separated crossing beneath the structure.

## 5) UTILITIES

Within the project corridor there are existing underground fiber optic, gas, electric, television, water, and sanitary sewer lines that parallel and cross $52^{\text {nd }}$ Ave $S$ in several locations. There are also overhead power lines that parallel the north and south right of way limits. Due to the lowering of the roadway as discussed above, it is anticipated underground utilities may be impacted as part of the project depending on existing depths. It is anticipated this alternative can be constructed with minimal to no impacts to the overhead power facilities.

## 6) PEDESTRIAN FACILITIES

This alternative will construct a 10 ' wide shared use path north and south of $52^{\text {nd }}$ Ave S . The shared use path will be separated by a grass boulevard up to 35 ' wide. At the intersections, the width of the grass boulevard will be reduced to a minimum of 6 ' to improve pedestrian crossing safety as well as vehicular sight distances. At grade north/south pedestrian crossings of $52^{\text {nd }}$ Ave $S$ will be provided at Sheyenne St, $63^{\text {rd }}$ St S, Veterans Boulevard and $45^{\text {th }}$ St S. A grade separated crossing will be provided along the west side of Drain 27 between $51^{\text {st }} \mathrm{St} \mathrm{S}$ and $47^{\text {th }} \mathrm{St} \mathrm{S}$. Connections will be established to existing pedestrian facilities that terminate at the project corridor. Additional grade separated crossing locations will be evaluated as part of the design phase of the project.

## 7) ENVIRONMENTAL IMPACTS

Within the corridor limits, ten wetlands and two Other Waters (OW) areas were identified. All wetlands were identified to be artificial ditch wetlands, while the other waters are the Sheyenne River and Drain 27. Due to elimination of the existing ditch system it is anticipated the project will permanently remove the existing wetlands created by shallow ditch grades. As these wetlands were identified as artificial, wetland mitigation is only anticipated for jurisdictional impacts greater than 0.10 acres. Mitigation requirements are pending a USACE jurisdictional determination.

It is anticipated this alternative can be constructed within the existing right of way. Temporary construction easements may be needed for sidewalk construction and grading near the right of way limits.

## 8) ALTERNATE INTERSECTION CONFIGURATIONS

Additional alternate intersection configurations are being considered to balance the operations of the corridor, environmental impacts and project cost while meeting the purpose and need of the project. The alternate configurations considered by intersection are:

```
| 63'rd StS
    - Roundabout
" 53rd Ave S
    - 3/4}\mathrm{ Access
    - Right In / RightOut
    - J-Turn
    - R-Cut
- Veterans Boulevard
    - Roundabout
- 51st AveS
    - 3/4 Access
    - Right In / RightOut
    - J-Turn
    - R-Cut
- 47 th St S
    - 3/4 Access
    - Right In / RightOut
    - J-Turn
    - R-Cut
```


## 1. $3 / 4 /$ ACCESS ALTERNATE INTERSECTIONCONFIGURATION

This alternate intersection configuration will restrict the thru and left turn movement along the local
roadways approaching $52^{\text {nd }}$ Ave S , reducing the number of intersection conflict points. However, due to the elimination of the thru movement as well as left turn movements from the approach roadways onto 52 nd, alternate routes may be needed. The $3 / 4$ access configuration is being considered at $53^{\text {rd }}$ Ave $S, 51^{\text {st }}$ Ave $S$ and $47^{\text {th }}$ St S. See Figure 3 for the proposed layout. The additional probable cost to construct this Alternate is $\$ 280,000$ per intersection compared to Alternative B.

Figure 3 - 3/4 Access IntersectionLayout


The typical section of this alternate will consist of two 11' wide eastbound and westbound thru traffic lanes separated by a 2' to $20^{\prime}$ wide median (see Figure 4). The increased median width, when compared to the full access intersection, is required to provide the additional left turn lane offset needed to construct the center island restricting the approach road thru and left turn movements. Left and right turn lanes will be implemented at major intersections as described under the base alternative.

Due to the wider median required to construct this alternate, the boulevard width separating $52^{\text {nd }}$ Ave S from the 10' wide shared use path north and south of the corridor will be reduced from approximately 35 ' to approximately $30^{\prime}$. All remaining components of the pedestrian facilities will match the basealternative.

Figure $4-3 / 4$ Access Intersection Typical Section

$\frac{\text { PROPOSED TYPICAL - } 3 / 4 \text { ACCESS }}{\text { Not } 10 \text { Sclule }}$

The $3 / 4$ access alternate results in an improved LOS for all approach legs of the 53 rd Avenue, $51^{\text {st }}$ Avenue and $47^{\text {th }}$ St intersections. The LOS listed below is for the worst leg of the approach roadway.

- $53^{\text {rd }}$ Avenue: B (am, WB Leg) A (pm)
- 51st Avenue: A (am) B (pm, SB Leg)
- 47 ${ }^{\text {th }}$ St S: $\quad \mathrm{A}(\mathrm{am}) \quad \mathrm{A}(\mathrm{pm})$

When compared to Alternative $B$, the $3 / 4$ access alternate intersection configuration will create significant additional travel time and distance for the approach roadway through and left turn movements. Both movements will need to travel to the next intersection to make a u-turn or take the local streets to a signalized intersection. The additional travel time will in many cases be greater than the delay encountered at the approach roadway under Alternative B.

With this alternate intersection configuration, the modifications to the Sheyenne River crossing will remain the same as described above under Alternative B. However, the additional median width required to construct the $3 / 4$ access will require an approximately 98 ' wide bridge deck as opposed to 90 '.

Existing utilities may require relocation as described above in Alternative B . The wider roadway section is anticipated to have minimal additional utility impacts when compared to the full access intersection. This alternate is also not anticipated to have any additional environmental or right of way impacts beyond the full access intersection alternate.

Right In / Right Out Alternate Intersection Configuration
The right in / right out alternate intersection configuration restricts all thru and left turn movements from the approach roadways as well as left turn movements from $52^{\text {nd }}$ Ave $S$. Similar to the $3 / 4$ access alternate, by eliminating the turning movements, the number of intersection conflict points can be reduced. However, alternate routes will be needed to access the local roadways. The right in / right out intersection configuration is being considered at $53^{\text {rd }} \mathrm{Ave} \mathrm{S}, 51^{\text {st }} \mathrm{Ave} \mathrm{S}$ and $47^{\text {th }} \mathrm{St} \mathrm{S}$. See Figure 5 for the proposed layout. The probable cost to construct this Alternate is $\$ 200,000$ per intersection less than AlternativeB.

Figure 5 - Right In / Right Out Intersection Layout


The typical section for this alternate consists of two 11' wide eastbound and westbound thru traffic lanes separated by a 4' to 15 ' wide raised median (see Figure 6). A 10 ' wide shared use path would be constructed north and south of $52^{\text {nd }}$ Ave $S$ separated by an approximately 35 ' wide boulevard. This configuration matches the typical section of the full access intersection.

Figure 6 - Right In / Right Out Intersection Typical Section

$\frac{\text { PROPOSED TYPICAL - RIGHT IN / RIGHT OUT }}{\text { NOT TO SCLE }}$

The right in / right out alternate results in an improved LOS for all approach legs of the $53^{\text {rd }}$ Avenue, $51^{\text {st }}$ Avenue and $47^{\text {th }}$ St intersections. The LOS listed below is for the worst leg of the approach roadway.

- $53^{\text {rd }}$ Avenue: A (am) A(pm)
- $51^{\text {st }}$ Avenue: A (am) B (pm, SB Leg)
- $47^{\text {th }}$ St S: $\quad$ A (am) B (pm, SB Leg)

When compared to Alternative B, the right in / right out alternate intersection configuration will create significant additional travel time and distance for the left turn movement from $52^{\text {nd }}$ Avenue $S$ as well as for the approach roadway through and left turn movements. These movements will need to travel to the next intersection to make a u-turn or take the local streets to a signalized intersection. The additional travel time will in many cases be greater than the delay encountered at the approach roadway under Alternative B.
$52^{\text {nd }}$ Ave $S$ will occupy the same footprint under this alternate intersection configuration as the full access intersection. Therefore, there will be no additional modifications to the structures crossing the Sheyenne River and Drain 27, utilities, wetlands, or right of way when compared to Alternative B.

Due to the access limitations of the Right In / Right Out alternate with minimal benefit to the corridor, this option is considered undesirable and unwarranted forthis corridor.

## J-TURN ALTERNATE INTERSECTIONCONFIGURATION

This alternate intersection configuration relocates the eastbound and westbound $52^{\text {nd }}$ Ave S left turn maneuvers approximately $500^{\prime}$ beyond the intersection. To complete a left turn maneuver from
$52^{\text {nd }}$ Ave $S$ to the side street, vehicles would perform a u-turn to access the approach roadway. A semi-circular full depth pavement section (loon) would be implemented to allow a WB-62 to complete the turning movement. The location of the left turn bay and loon would be designed to provide a minimum of 50 ' separation between the loon and $52^{\text {nd }}$ Ave $S$ right turn lane. As the thru and left turn movements from the approach roadways would also be restricted, the left turn bay along $52^{\text {nd }}$ Ave S would be extended through the intersection. The longer turn lane bay allows for northbound to westbound, southbound to eastbound, northbound thru and southbound thru traffic to cross two $52^{\text {nd }}$ Ave S thru lanes and enter the turn lane directly. This eliminates the weaving maneuvers required to complete the desired turning movements. This alternate results in decreased intersection conflict points when compared to a full access intersection, with minimal access restrictions to the local roadways. The J-Turn alternate intersection configuration is being considered at $53^{\text {rd }}$ Ave $\mathrm{S}, 51^{\text {st }}$ Ave S, and $47^{\text {th }}$ St S. Due to the spacing of $53^{\text {rd }}$ Ave $S$ and Veterans Boulevard, the $52^{\text {nd }}$ Ave $S$ eastbound left turn movement at $53^{\text {rd }}$ Ave $S$ would be restricted. See Figure 7 for the proposed layout. The additional probable cost to construct this Alternate is $\$ 70,000$ per intersection compared to Alternative B.

Figure 7 - J-Turn Intersection Layout


The typical section consists of two 11' wide eastbound and westbound thru traffic lanes separated by a $12^{\prime}$ to 36 ' wide median (see Figure 8). To allow for a WB-62 to complete the u-turn maneuver within the J-Turn, a loon will be constructed. The loon utilizes approximately a 57’ radius and widens the adjacent roadway up to $30^{\prime}$. A narrower and wider median option were evaluated when determining the appropriate width for this alternate.

Figure 8 - J-Turn Intersection Typical Section


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The narrower median option maintained the 4' to 15 ' wide median matching the full access intersection configuration. However, to accommodate the wheel path of a WB-62, the loon widened the adjacent roadway by up to 60'. Due to the amount of widening, a mountable curb and truck apron were implemented to delineate the thru roadway from the turning movement (see Figure 9). The $52^{\text {nd }}$ Ave S right turn lane was extended tothe loon and striped as a drop lane to allow acceleration prior to entering the thru traffic lanes. This option was eliminated from consideration due to potential driver confusion associated with left turning traffic leaving the roadway and the associated weaving maneuver required to reenter the thru traffic lanes.

Figure 9 - J-Turn Intersection Typical Section, Narrow Median

$\frac{\text { PROPOSED TYPICAL - J-TURN NARROW MEDIAN }}{\text { MOT TO SCNE }}$

The wider median option utilized a $32^{\prime}$ to $56^{\prime}$ ' wide median, minimizing the additional widening needed to accommodate a WB-62. With this option, the loon widened the adjacent roadway by approximately $16^{\prime}$. However, the additional roadway widening resulted in boulevard widths of 2 ' adjacent to the 10 ' wide shared use path to minimize right of way acquisition. As the narrow boulevard provides minimal separation of pedestrian and vehicular traffic it does not meet the desire of the project to serve as an important link in the multimodal transportation system. Therefore, this option was eliminated from consideration. See Figure 10 for the proposed J-Turn typical section with wide median.

Figure 10 - J-Turn Intersection Typical Section, Wide Median


PROPOSED TYPICAL - J-TURN WIDE MEDIAN

The J-Turn alternate results inan improved LOS for all approach legs of the $53^{\text {rd }}$ Avenue, $51^{\text {st }}$ Avenue and $47^{\text {th }}$ St intersections. The LOS listed below is for the worst leg of the approach roadway.

- $53^{\text {rd }}$ Avenue: A (am) A(pm)
- $51^{\text {st }}$ Avenue: A (am) B (pm, SB Leg)
- $47^{\text {th }}$ St S: A (am) A(pm)

When compared to Alternative B, the J-Turn alternate intersection would increase corridor travel time and distance since the left turn from $52^{\text {nd }}$ Avenue as well as minor street through and left turn movements need to travel about 1,000 additional feet to complete the maneuver. The additional distance is anticipated to increase travel times by approximately 25 seconds for the approach traffic. For the left turn movement from $52^{\text {nd }}$ Avenue $S$, the increased travel time will be greater than the delay for the left turn movement under Alternative B. However, the increased travel time is more than offset by the reduced delay encountered at the approach roadway.

Previous studies completed on J -Turn intersections indicated that the J -Turn design results in approximately a $35 \%$ reduction in overall crash rate with approximately $50 \%$ reduction in the crash rate for injury and fatal crashes when compared to a full access intersection.

Under this alternate, the corridor width will remain unchanged at the Sheyenne River crossing when compared to the full access interchange. Therefore, no additional modifications will be required to the structure. To accommodate the J -Turn at $51^{\text {st }}$ Ave S and $47^{\text {th }} \mathrm{St} \mathrm{S}$, the median width crossing Drain 27 will be widened to $36^{\prime}$. Due to the additional widening, it is anticipated two separate structures, each approximately 40 ' wide, would be constructed for eastbound and westbound traffic in order to reduce project costs related to the superstructure and substructure.

The additional roadway width is also likely to increase the utility relocations required throughout the corridor. In addition to impacts to the underground facilities depending on existing cover, this option is likely to impact the overhead power distribution lines that parallel the corridor. The J-Turn intersection configuration is not anticipated to increase wetland impacts. However, permanent right of way acquisition, approximately $15^{\prime}$ wide and 100 ' long, are anticipated for the construction of the loons at each location.

Due to the access limitations when compared to the full access and R-Cut alternate intersection
configurations, the J-Turn was determined to be an undesirable intersection option for this corridor.

## R-CUT ALTERNATEINTERSECTIONCONFIGURATION

The R-Cut (Reduced Conflict U-Turn) alternate intersection configuration is similar to the J-Turn alternate. However, a $3 / 4$ access is provided at the intersection to allow eastbound and westbound $52^{\text {nd }}$ Ave S left turn movements to side streets. Under this alternate, the J-Turn will be utilized for northbound to westbound, southbound to eastbound, northbound thru and southbound thru movements only. As the J-Turn will be utilized by vehicles entering from the approach roadway only, the turn lanes will be extended to the $3 / 4$ access, the taper length decreased, and the approach roadway radius increased to allow approach traffic to enter the turn lane by crossing the two $52^{\text {nd }}$ Ave S thru lanes (see Figure 11). Thus, eliminating the weaving maneuver and potential conflict points along $52^{\text {nd }}$ Ave $S$. This alternate intersection is being considered at $53^{\text {rd }}$ Ave $S, 51^{\text {st }}$ Ave $S$ and $47^{\text {th }}$ St S. The additional probable cost to construct this Alternate is $\$ 230,000$ per intersection compared to Alternative B.

Figure 11 - R-Cut Intersection Layout


Similar to the J-Turn alternate intersection configuration, the typical section will consist of two 11' wide eastbound and westbound thru lanes separated by a 12 ' to 36 ' wide median. To allow for a WB-62 to complete the u-turn maneuver within the J-Turn, a 57' radius loon will be constructed. The loon widens the adjacent roadway up to 30 '. See Figure 12 for the proposed typical section. As was done for the JTurn alternate, a narrower and wider median option were also evaluated when determining the appropriate width for this alternate.

Figure 12 - R-Cut Intersection Typical Section


PROPOSED TYPICAL-R-CUT

The R-Cut alternate results inan improved LOS for all approach legs of the $53^{\text {rd }}$ Avenue, $51^{\text {st }}$ Avenue and $47^{\text {th }}$ St intersections. The LOS listed below is for the worst leg of the approach roadway.

- $53^{\text {rd }}$ Avenue: B (am, WB Leg) A (pm)
- 51 ${ }^{\text {st }}$ Avenue: A (am) B (pm, SB Leg)
- $47^{\text {th }}$ St S: $\quad \mathrm{A}(\mathrm{am}) \quad \mathrm{A}(\mathrm{pm})$

When compared to Alternative B, the R-Cut alternate intersection would increase corridor travel time and distance since the minor street through and left turn movements need to travel about 1,000 additional feet to complete the maneuver. The additional distance is anticipated to increase travel times by approximately 25 seconds for the approach traffic. However, the increased travel time is more than offset by thereduced delay encountered at the approach roadway.

The Federal Highway Crash Modification Factors Clearinghouse has several studies that indicate an R-Cut intersection will reduce the crash rate when compared to a full access intersection by approximately $30 \%$ as well as reduce the overall severity of the crashes.

Due to the $36^{\prime}$ wide median at the Drain 27 crossing, it is anticipated the $90^{\prime}$ wide single structure will be replaced with two separate 40 ' wide structures for eastbound and westbound traffic. At the Sheyenne River crossing, the roadway width will remain unchanged when compared to Alternative B. Therefore, no change to the $90^{\prime}$ wide structure described in Alternative B is anticipated.

As the R-Cut alternate will occupy the same footprint as the J-Turn, additional utility and right of way impacts are anticipated when compared to the full access intersection. Due to the wider median and loons, impacts to the overhead power distribution lines are anticipated. In addition, permanent right of way acquisition, approximately $15^{\prime}$ wide and 100 ' long are anticipated for the construction of the loons.

## 2. ROUNDABOUT ALTERNATEINTERSECTION CONFIGURATION

This alternate intersection configuration replaces the signalized intersections at $63^{\text {rd }} \mathrm{St} \mathrm{S}$ and Veterans Boulevard with multilane roundabouts. The roundabout provides two eastbound and westbound thru lanes with a single northbound and southbound thru lane and consists of a 180' diameter inscribed circle, 96 ' diameter center island and 8 ' wide mountable truck apron. The roundabout approach and exit legs will be designed to meet current design practices to achieve the desired approach speed and avoid vehicular path overlap. At grade pedestrian facilities as well as east/west and north/south crossings will be maintained within the intersection. Due to the size of inscribed circle, additional environmental and right of way impacts are anticipated when compared to a signalized intersection. See Figures 13 and 14 for the proposed layout and typical section. The probable cost to construct this Alternate is $\$ 160,000$ per intersection less than Alternative B due to reduced pavement area within the intersection.

Figure 13 - Roundabout Intersection Layout


Figure 14 - Roundabout Intersection Typical Section


The roundabout alternate results in an improved LOS for the 63 ${ }^{\text {rd }}$ St and Veterans Boulevard intersections. The $63{ }^{\text {rd }}$ Street intersection is expected to operate at a LOS A during the am and pm peak hour. At Veterans Boulevard, the intersection is expected to operate at a LOS A during the am and pm peak hour.

Generally, the roundabout alternate operates similar to Alternative B. The roundabouts are expected to slightly decrease delay and increase the LOS at 63rd Street and Veterans Boulevard. Intersection alternates described above for the approach roadways at $53^{\text {rd }}$ Avenue, $51^{\text {st }}$ Avenue, and $47^{\text {th }}$ Street are expected to operate similarly whether a signalized intersection or roundabout are utilized at $63^{\text {rd }}$ Street and Veterans Boulevard. Corridor travel times are expected to be approximately 9 seconds longer ( 289.5 seconds, with average speed of 29 mph ) with the roundabout alternate.

According to the most recent studies, a 2-lane by 1-lane roundabout has about the same crash rate as a signalized intersection. However, the fatality and serious injury crash rates for this roundabout configuration is approximately $50 \%$ less than a signalized intersection. This is based on the Federal Highway Crash Modification Factors Clearinghouse and consistent with a recently completed study of traffic safety at roundabouts in Minnesota.

As this alternate will not impact the corridor outside of the $63{ }^{\text {rd }} \mathrm{St} \mathrm{S}$ and Veterans Boulevard intersections, modifications to the Sheyenne River and Drain 27 crossing are anticipated to match those described under Alternative B. It is anticipated both structures will be replaced or modified to provide a 90 ' wide bridge deck.

As the roundabout increases the footprint of the $63{ }^{r d} \mathrm{St} S$ and Veterans Boulevard intersections when compared to a signalized intersection, it is anticipated this alternate will result in increased utility impacts to the overhead power distribution lines. In addition, permanent right of way acquisition is anticipated within each quadrant of the intersections.

See Table 1 for a summary of each alternate intersection configuration.

## HOUSTON

ENGINEERING INC.

| Location | Traffic Operations |  |  |  |  | Crashes |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Intersection LOS | Access | Averare Sneed (MPH) | Intersection <br> Delav (Sec) | Additional Travel Distance (Ft) | Rate | Severity | Construction Cost (In Addition to Alt B) |
| Sheyenne St |  |  |  |  |  |  |  |  |
| Roundabout | A | Full | 31 | 9 | - | Unchanged | Reduced 50\% | - |
| McMahon Estates Circle |  |  |  |  |  |  |  |  |
| Full Access | - | Full | - | - | - | - | - | - |
| Right In / Right Out | - | Limited | - | - | - | Reduced 30\% | Reduced | -\$200,000.00 |
| $68^{\text {th }} \mathrm{St} \mathrm{S}$ |  |  |  |  |  |  |  |  |
| Full Access | - | Full | - | - | - | - | - | - |
| $63^{\text {rd }}$ St S |  |  |  |  |  |  |  |  |
| Signalized | B | Full | 32 | 18 | - | - | - | - |
| Roundabout | A | Full | 31 | 9 | - | Unchanged | Reduced 50\% | -\$160,000.00 |
| $53^{\text {rd }}$ Ave S |  |  |  |  |  |  |  |  |
| Full Access | E | Full | - | 16 | - | - | - | - |
| 3/4 Access | B | Partial | - | 6 | 2,000-3,000 | Reduced 30\% | Reduced | \$280,000.00 |
| Right In / Right Out | A | Limited | - | 8 | 2,000-3,000 | Reduced | Reduced | -\$200,000.00 |
| J-Turn | A | Full | - | 9 | 600 | Reduced 35\% | Reduced 50\% | \$70,000.00 |
| R-Cut | B | Full | - | 6 | 600 | Reduced 30\% | Reduced | \$230,000.00 |
| Veterans Blvd |  |  |  |  |  |  |  |  |
| Signalized | B | Full | 32 | 12 | - | - | - | - |
| Roundabout | A | Full | 31 | 5 | - | Unchanged | Reduced 50\% | -\$160,000.00 |
| $53^{\text {rd }}$ St S |  |  |  |  |  |  |  |  |
| Full Access | F | Full | - | 182 | - | - | - | - |
| 3/4 Access | B | Partial | - | 5 | 3,000-6,000 | Reduced 30\% | Reduced | \$280,000.00 |
| Right In / Right Out | B | Limited | - | 11 | 3,000-6,000 | Reduced | Reduced | -\$200,000.00 |
| $J$-Turn | B | Full | - | 11 | 600 | Reduced 35\% | Reduced 50\% | \$70,000.00 |
| R-Cut | B | Full | - | 5 | 600 | Reduced 30\% | Reduced | \$230,000.00 |
| $47^{\text {th }} \mathrm{St}$ S |  |  |  |  |  |  |  |  |
| Full Access | F | Full | - | 47 | - | - | - | - |
| 3/4 Access | A | Partial | - | 5 | 3,000-6,000 | Reduced 30\% | Reduced | \$280,000.00 |
| Right In / Right Out | B | Limited | - | 10 | 3,000-6,000 | Reduced | Reduced | -\$200,000.00 |
| $J$-Turn | A | Full | - | 9 | 600 | Reduced 35\% | Reduced 50\% | \$70,000.00 |
| R-Cut | A | Full | - | 5 | 600 | Reduced 30\% | Reduced | \$230,000.00 |
| $45^{\text {th }} \mathrm{St} \mathrm{S}$ |  |  |  |  |  |  |  |  |
| Signalized | C | Full | 32 | 26 | - | - | - | - |


[^0]:    ${ }^{1}$ Temporary signals were recently installed at the intersection of 63rd Street and 52 nd Avenue. However, traffic volumes and operational data presented in this memo were collected prior to installation of these temporary signals and therefore reflect operations for a side stop-controlled intersection at this location.

[^1]:    ${ }^{2}$ Some features associated with the Active Living Streets plan may not be feasible as part of this project. For example, land use design guidelines such as setbacks or building orientation are outside of the scope of this project. Similarly, as the corridor does not have existing transit service, consideration of dedicated transit lanes may not be appropriate.

